

TRAFFIC IMPACT & ACCESS STUDY

Proposed Residential Development Project

CENTURY MILL ESTATES

Bolton, Massachusetts

Applicant

CENTURY MILL ESTATES, LIMITED PARTNERSHIP

September 8, 2005

prepared by

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Dermot J. Kelly Associates, Inc. (DJK) has conducted a Traffic Impact and Access Study for a proposed residential development project to be known as Century Mill Estates located between and adjacent to Century Mill Road and Spectacle Hill Road in Bolton, Massachusetts. Figure 1 graphically presents the site location in relation to the local and regional roadway network. This report identifies existing traffic operating parameters on the adjacent roadway system, evaluates the potential traffic-related impacts associated with the proposed project and provides traffic-related recommendations, which would minimize the potential impacts of the proposed project.

Based on the traffic analysis as presented in this study, there is sufficient capacity throughout the study area to accommodate the proposed residential development project. The overall traffic-related impacts will not create any adverse impacts on traffic operations that would require offsite roadway improvements.

PROJECT DESCRIPTION

The project proponent presently is planning for the construction of 78 single family homes. Current project planning calls for the site to be accessed at two locations, one along Century Mill Road and one along Spectacle Hill Road. The North Site Drive would intersect Century Mill Road at a right angle forming a "T" type intersection. Similarly, the West Site Drive would intersect Spectacle Hill Road at a right angle.

ALTERNATIVE STUDIES

For the purposes of this report, the following alternatives were evaluated:

- *No-Build* - The No-Build alternative was examined to establish the 2010 baseline traffic-volume conditions. The incremental impacts of the proposed project may be determined by making comparisons to the No-Build alternative. The No-Build alternative assumes that the project is not built.

- *Build* - The Build alternative includes the development of the proposed project. The Build alternative is evaluated in this report and is compared to the No-Build analysis conditions. The Build condition represents the increase in traffic from the previous condition to the proposed condition (i.e., the delta impact). The Build condition includes the construction of the proposed residential development project.

STUDY AREA

The study area includes the following intersections:

- Century Mill Road at proposed North Site Drive,
- Spectacle Hill Road at the proposed West Site Drive, and
- Hudson Road (Route 85) at Century Mill Road.

EXISTING TRAFFIC VOLUME CONDITIONS

Analysis of the recent traffic counts indicates that an average of 767 vehicles per day (vpd) were observed along Century Mill Road adjacent to the site during January 2004. During the two peak travel demand periods, which generally occurred between 7 and 9 AM and between 4 and 6 PM, an average of 107 and 95 vehicles per hour (vph) were observed along Century Mill Road during the weekday morning and evening peak hours, respectively. Similarly, along Spectacle Hill Road the observed average daily traffic was 243 vpd with 21 vph observed during the morning peak hour and 25 vph observed during the evening peak hour.

FUTURE TRAFFIC VOLUME CONDITIONS

Background Traffic Growth

To represent future 2010 No-Build traffic-volume conditions, the existing traffic volumes were increased to account for two other development projects in Hudson and general background traffic growth. The two other projects are located along Central Street and include 150 apartments and 30 single family homes. The existing traffic volumes were increased by +2.0 percent per year up through 2010.

Site-Generated Traffic

Based on the Institute of Transportation Engineers (ITE) trip generation rates for single family homes, it is anticipated that 78 single family homes would generate approximately 828 vehicle-trips per day (vpd). This daily volume would be split evenly with 414 vehicle-trips entering and 414 vehicle-trips exiting the site over the course of the entire 24-hour day.

More importantly, during the weekday morning peak hour, 78 residences would generate a total of 64 vehicle-trips with 16 vehicle-trips entering and 48 vehicle-trips exiting the project site. Similarly, during the evening peak hour, it is anticipated that 86 vehicle-trips would be generated. The 86 vehicle-trips would consist of 54 vehicle-trips entering and 32 vehicle-trips exiting the project parcel during the peak 60-minute period. The remaining vehicle-trips would occur over the course of the day.

Trip Distribution

Trip-distribution patterns were developed specifically for this study and are summarized as follows:

At the North Drive:	
• Century Mill Road to/from the East	55%
• Century Mill Road to/from the West	15%
At the West Drive:	
• Spectacle Hill Road to/from the North	10%
• Spectacle Hill Road to/from the South	20%
TOTAL	
	100%

TRAFFIC OPERATION ANALYSIS

Unsignalized intersection capacity analysis was performed for the study area intersection. The capacity analysis is summarized below by location.

Century Mill Road at the Proposed North Site Drive

Under 2010 Build with development traffic volume conditions, the left- and right-turns exiting the site and the left-turns entering the site would operate at Level of Service (LOS) A during both the morning and evening peak hours.

Spectacle Hill Road at the Proposed West Site Drive

Similar to the Proposed North Site Drive intersection, the left-turns entering the site and the left- and right-turns exiting the site would operate at LOS A during both the morning and evening peak hours under 2010 Build with development traffic volume conditions.

Hudson Road at Century Mill Road

The left-turns movement on Hudson Road will operate at LOS A up through the 2010 Build traffic volume conditions. The left- and right-turns on Century Mill Road will operate at LOS B during both the morning and evening peak hours up through 2010 Build with development traffic volume conditions.

RECOMMENDATIONS

The final phase of the analysis process is to make traffic-related recommendations that would minimize the impact of the project on the adjacent transportation system. The proponent has made a commitment to implement all recommendations listed below subject to approval, construction and the eventual occupation of the proposed project:

- The Proposed North and West Site Drive intersections at Century Mill Road and Spectacle Hill Road are suggested to consist of a minimum 20-foot wide cross section. Intersection corner radii are suggested to be 5- to 30-foot radius. The current proposal for the North and West Site Drives meets or exceeds the above recommendation.

- It is suggested that the North Site Drive and the West Site Drive approaches to Century Mill Road and Spectacle Hill Road be placed under STOP sign control with a painted stop line, respectively. All internal intersections are suggested to be appropriately placed under stop sign control.

- In order to enhance the available safety sight lines at the Proposed Site Drive intersections with both Century Mill Road and Spectacle Hill Road, topographic ground elevations, landscaping, signage (if any), etc. are suggested to be maintained such that adequate sight distances are available throughout the life of the project. The sight distance maintenance triangular area along the property frontage should extend 14.4 feet (4.4 meters) back into the site drives from the existing edge of the roadway (representing an exiting vehicle) and extend to a maximum possible location (338 feet minimum along Century Mill Road to the east; 349 feet to the west along Century Mill Road; and 305 feet minimum to the north and south along Spectacle Hill Road) representing an approaching vehicle in each direction along Century Mill Road and Spectacle Hill Road, respectively.

- Intersection advanced warning signs are suggested to be placed along both Century Mill Road and Spectacle Hill Road in advance of the proposed access/egress driveways.

SUMMARY/CONCLUSION

Based on the traffic analysis as presented in this study, there is sufficient capacity throughout the study area to accommodate the proposed residential development project. The overall traffic-related impacts will not create any adverse impacts on traffic operations that would require offsite roadway improvements.

PROJECT DESCRIPTION

PROPOSAL

The project proponent presently is planning for the construction of 78 single family homes. Current project planning calls for the site to be accessed at two locations, one along Century Mill Road and one along Spectacle Hill Road. The North Site Drive would intersect Century Mill Road at a right angle forming a "T" type intersection. Similarly, the West Site Drive would intersect Spectacle Hill Road at a right angle.

ALTERNATIVES TO THE PROJECT

For the purposes of this report, the following alternatives were evaluated:

- *No-Build* - The No-Build alternative was examined to establish the 2010 baseline traffic-volume conditions. The incremental impacts of the proposed project may be determined by making comparisons to the No-Build alternative. The No-Build alternative assumes that the project is not built.
- *Build* - The Build alternative includes the development of the proposed project. The Build alternative is evaluated in this report and is compared to the No-Build analysis condition. The Build condition represents the increase in traffic from the previous condition to the proposed condition (i.e., the delta impact). The Build condition includes the construction of the proposed residential development project.

THE EXISTING CONDITIONS

FIELD SURVEY

A comprehensive field inventory of the adjacent roadway system was conducted during December 2003 and January 2004. The field inventory included collection of existing roadway geometrics, traffic volumes, and safety data for the adjacent roadways in the vicinity of the site.

Traffic volumes were measured by means of two Automatic Traffic Recorder (ATR) counters and two manual intersection Turning Movement Counts. Safety inventories included vehicle speed observations and a detailed evaluation of the safety sight lines at the proposed site drive locations along Century Mill Road and Spectacle Hill Road.

GEOMETRICS

Century Mill Road

Century Mill Road is generally a two-way, east/west roadway connecting Hudson Road to the east with South Bolton Road to the west. Century Mill Road is approximately 16 feet wide in the immediate vicinity of the site with no pavement markings or speed limit signs noted during the field inventory period. Century Mill Road approaches the proposed site drive over a relatively level/tangent alignment with a winding alignment to the east and west.

Spectacle Hill Road

Spectacle Hill Road is generally a north/south roadway connecting South Bolton Road and Century Mill Road to the north with Bolton Street and Central Street to the south. Spectacle Hill Road is a bi-directional roadway approximately 17 feet wide. No pavement markings or speed limit signs were noted during the field inventory period. The roadway alignment is generally tangent in the immediate vicinity of the Proposed West Site Drive. To the south of the Proposed West Site Drive, Spectacle Hill Road slopes down in the northbound direction at a rate of approximately 6% with a large radius curve located to the south. To the north, Spectacle Hill Road is generally level.

Century Mill Road at Hudson Road

Century Mill Road intersects Hudson Road at a right angle forming a "T" type intersection. The Century Mill Road approach to the intersection is under stop sign control with both a painted stop line and the painted word "STOP." Hudson Road is painted with a double yellow centerline located to the north and south with a single yellow centerline located opposite Century Mill Road.

TRAFFIC VOLUMES

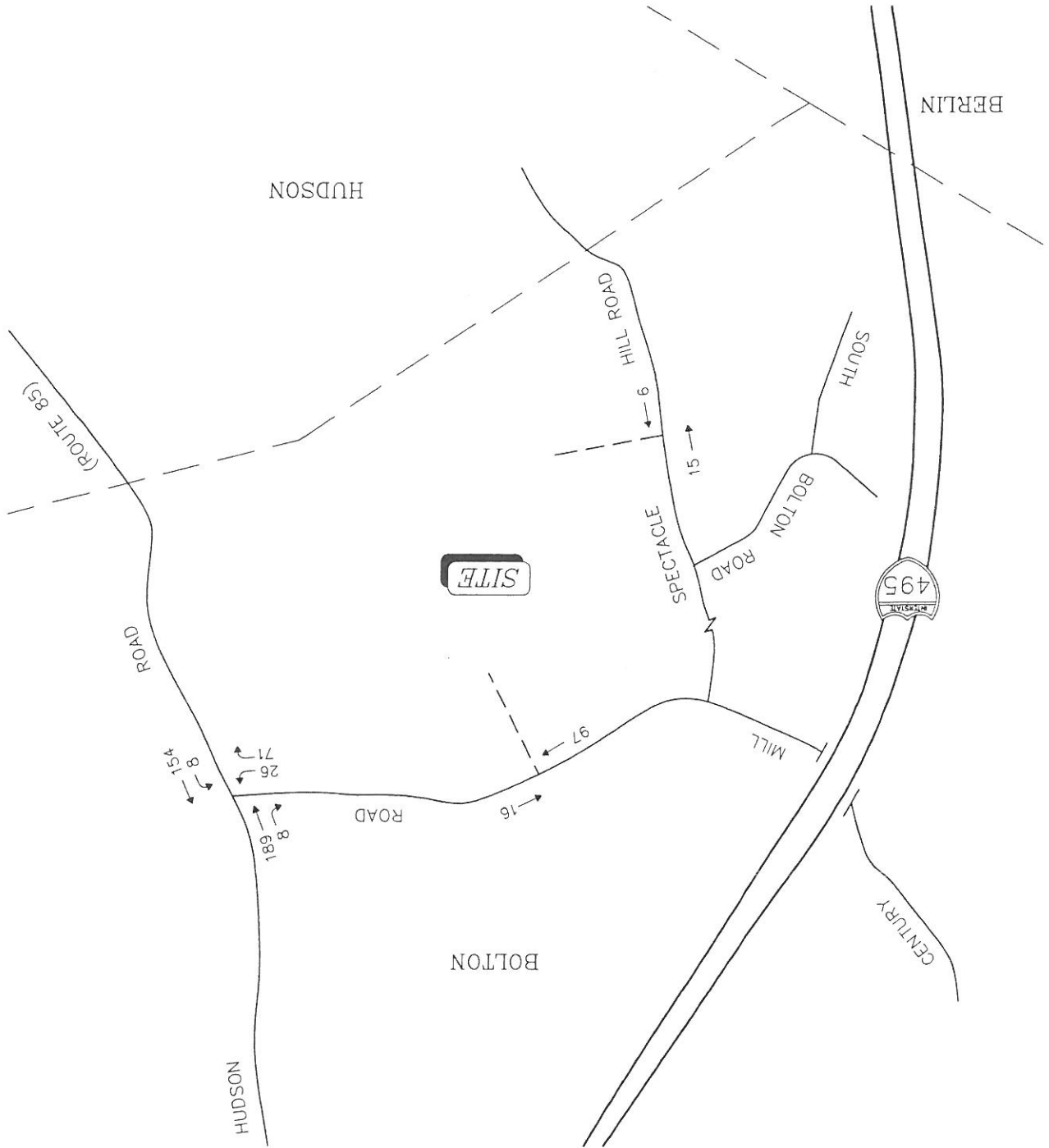
Existing traffic volumes were recorded mechanically by Automatic Traffic Recorder (ATR) counters over a 48-hour period during Wednesday and Thursday, January 7 and 8, 2004. The ATR data was substantiated by actual manual intersection Turning Movement Counts (TMC) conducted on Wednesday, January 7, 2004. All traffic volume data is reproduced in the Appendix of this study. These traffic volumes were reviewed to determine average daily and peak-hour traffic volumes along Century Mill Road and Spectacle Hill Road for this study. Table 1 summarizes the 2004 ATR traffic-volume data collected for this study.

Analysis of the recent traffic counts indicates that an average of 767 vehicles per day (vpd) were observed along Century Mill Road adjacent to the site during January 2004. During the two peak travel demand periods, which generally occurred between 7 and 9 AM and between 4 and 6 PM, an average of 107 and 95 vehicles per hour (vph) were observed along Century Mill Road during the weekday morning and evening peak hours, respectively. Similarly, along Spectacle Hill Road the observed average daily traffic was 243 vpd with 21 vph observed during the morning peak hour and 25 vph observed during the evening peak hour.

The Century Mill Road traffic volumes represent a composite result of the ATR and TMC data. The Spectacle Hill Road traffic volumes represent the results of the ATR data.

To represent 2005 existing traffic volume conditions, the 2004 traffic volumes were increased by 2% to reflect traffic growth between 2004 and 2005. Figures 2 and 3 graphically present the results of the traffic volume inventory for the weekday morning and evening peak hours.

Figure 2: 2005 Existing
Weekday Morning
Peak Hour Traffic Volumes



↓ Schematic

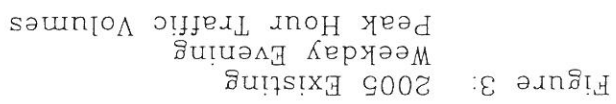


TABLE 1
EXISTING TRAFFIC VOLUME SUMMARY

Location/ Time Period	Daily Traffic (24 hour) (vpd) ^a	Peak Hour	Peak-Hour Traffic Volume (vph) ^b	Factor ^c K
Century Mill Road adjacent to the Proposed North Site Drive:				
Wednesday 1/7/04	799	7:15-8:15 AM	104	13.0
Thursday 1/8/04	735	7:15-8:15 AM	78	15.0
Average Weekday	767	Morning Peak	107	14.0
		Evening Peak	95	12.0
Spectacle Hill Road adjacent to the Proposed West Site Drive:				
Wednesday 1/7/04	262	7:15-8:15 AM	21	8.0
Thursday 1/8/04	224	7:30-8:30 AM	21	9.4
Average Weekday	243	Morning Peak	21	8.6
		Evening Peak	25	10.3

^aVehicles per day.
^bVehicles per hour.
^cPercent of daily traffic occurring during the peak hour.

VEHICLE SPEEDS

Spot speeds were recorded by automatic traffic recorders (ATR) placed along Century Mill Road and Spectacle Hill Road over a 48-hour period. The results of the speed measurements are summarized in Table 2. As shown, the mean speed along Century Mill Road varied between 36 and 38 mph. The 85th percentile speed was recorded at 42, 43 and 44 mph. The 10 mph pace speed was generally recorded at 35-44 mph for both the eastbound and westbound direction of travel. Along Spectacle Hill Road, the mean speed varied between 28 and 32 mph with the 85th percentile speed recorded at 35 mph northbound and between 38 and 39 mph southbound. The pace speed was recorded at 25-34 mph for the northbound direction and between 26-35 mph and 30-39 mph southbound. The actual data is reproduced in the appendix of this report.

TABLE 2
VEHICLE SPEED SUMMARY

Location/ Direction/ of Travel	Official Speed Limit ^a (mph)	Mean 50 th Percentile Speed (mph)	85 th Speed Percentile (mph)	10 mph Speed Pace (mph)	Total Number of Observations
<i>Century Mill Road adjacent to the Proposed North Site Drive:</i>					
Eastbound	40	36	42	30-39	398
Wednesday, 1/7/04	40				
Thursday, 1/8/04	40	38	44	35-44	363
Total	--	--	--	--	761
Westbound	40	37	43	35-44	401
Wednesday, 1/7/04	40				
Thursday, 1/8/04	40	37	43	35-44	372
Total	--	--	--	--	773
<i>Spectacle Hill Road adjacent to the Proposed West Site Drive:</i>					
Northbound	40	30	35	25-34	126
Wednesday, 1/7/04	40				
Thursday, 1/8/04	40	28	35	25-34	96
Total	--	--	--	--	222
Southbound	40	31	38	30-39	136
Wednesday, 1/7/04	40				
Thursday, 1/8/04	40	32	39	26-35	128
Total	--	--	--	--	264

^aBased on information provided by the Bolton Police Department.
^bmph = miles per hour.

INTRODUCTION

This section of the report determines the future traffic volume levels along the study area roadways and intersections. To determine the impact of site-generated traffic volumes on the roadway network under future conditions, the existing traffic volumes in the study area were projected to the year 2010 to be consistent with Massachusetts State guidelines for Traffic Impact Assessments. Traffic volumes on the roadway network at that time would include all existing traffic, background traffic growth, and the site-generated traffic volumes.

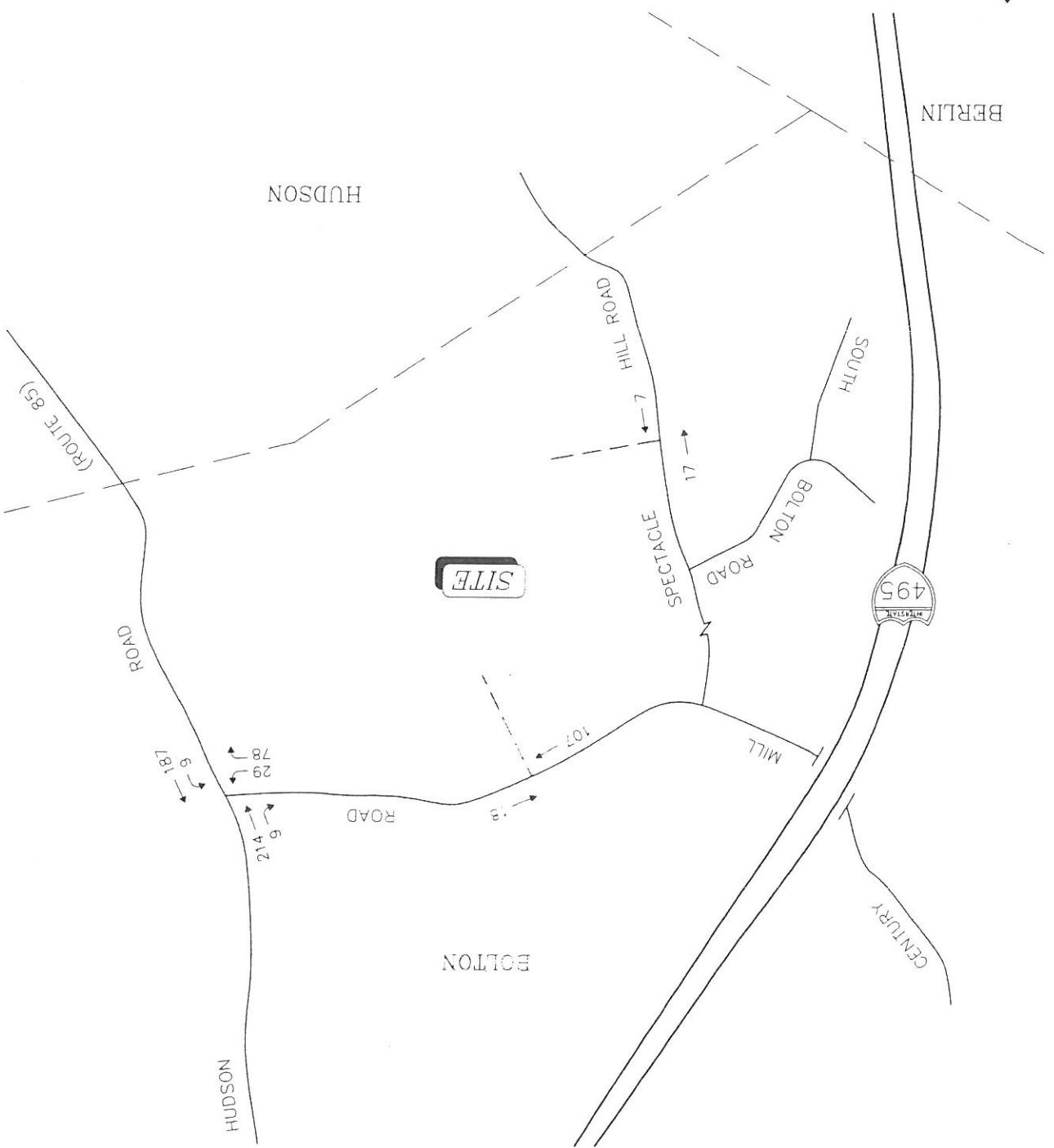
BACKGROUND TRAFFIC GROWTH

Traffic growth on area roadways is a function of the expected land development in the immediate area as well as the surrounding region. Several methods can be used to estimate this growth. A procedure frequently employed is to identify the location and type of new developments planned to occur during the designated planning horizon, estimate the traffic to be generated and assign it to the area roadway network. This method usually produces a realistic estimate of growth for local traffic.

The Planning Office for the Town of Bolton was consulted during the preparation of this Study to identify existing and projected development projects anticipated to occur by 2010. Based on this information and other background information, the existing traffic volumes were increased to account for 150 apartments and 30 single family homes to be located along Central Street in Hudson and by +2.0 percent per year up through 2010.

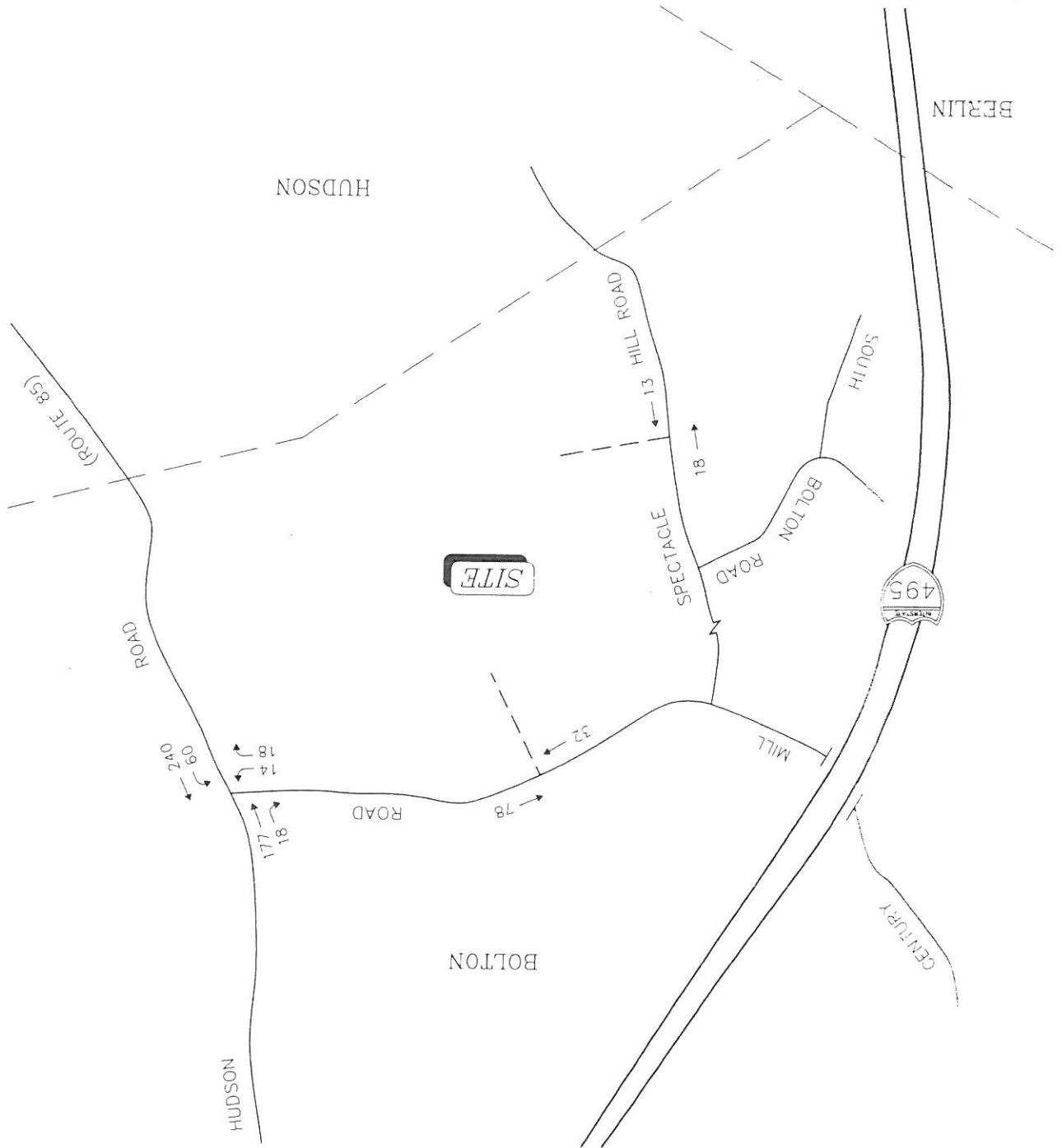
Figures 4 and 5 graphically present the 2010 No Build weekday morning and evening peak-hour traffic volumes.

Figure 4: 2010 No Build
Weekday Morning
Peak Hour Traffic Volumes



↓
Schematic

Figure 5: 2010 No Build
Weekday Evening
Peak Hour Traffic Volumes



VEHICLE-TRIP GENERATION

Traffic volumes generated by residential developments normally follow well-established patterns with respect to magnitude and temporal distribution. Measurements of numerous such developments published by the Institute of Transportation Engineers (ITE) have established trip generation rates, which have been standardized for analysis purposes. The rate at which residential projects generate vehicle-trips depends on the type of residential project; i.e., single-family homes, duplexes versus condominiums, age-restricted residences, etc.

Based on the Institute of Transportation Engineers (ITE) trip generation rates for single family homes, it is anticipated that 78 residences would generate approximately 828 vehicle-trips per day (vpd). This daily volume would be split evenly with 414 vehicle-trips entering and 414 vehicle-trips exiting the site over the course of the entire 24-hour day.

More importantly, during the weekday morning peak hour, the proposed project would generate a total of 64 vehicle-trips with 16 vehicle-trips entering and 48 vehicle-trips exiting the project site. Similarly, during the evening peak hour, it is anticipated that 86 vehicle-trips would be generated. The 86 vehicle-trips would consist of 54 vehicle-trips entering and 32 vehicle-trips exiting the project parcel during the peak 60-minute period. The remaining vehicle-trips would occur over the course of the day.

Table 3 summarizes the peak-hour and daily project-generated traffic volumes for the proposed 78 single family homes.

TABLE 3
VEHICLE-TRIP GENERATION SUMMARY ^a

Time Period/ Direction of Travel	78 Single Family Homes			
Weekday Morning	Entering (vph) ^b	16	48	64
	Exiting (vph)			
	Total (vph)			
Weekday Evening	Entering (vph)	54	32	86
	Exiting (vph)			
	Total (vph)			
Average Weekday	Entering (vpd) ^c	414	414	828
	Exiting (vpd)			
	Total (vpd)			

^aSource: Institute of Transportation Engineers (ITE) *Trip Generation*, 7th Edition, 2003, Land Use Code 210 Single Family Homes.
^bvph = vehicle-trips per hour.
^cvpd = vehicle-trips per day.

TRAFFIC DISTRIBUTION/ASSIGNMENT

Directional distribution of generated trips to and from the proposed development is expected to follow existing traffic patterns which, in turn, is a function of population densities, shopping opportunities, areas of employment and recreational activities. Accordingly, the directional split of the new trips originating from, or destined to, the development was based on existing traffic patterns as observed for this study. Table 4 summarizes the traffic distribution and assignment of site-generated trips.

The site-generated traffic volumes were superimposed onto the 2010 No Build traffic volume networks creating the 2010 Build traffic volume networks. Figures 6 and 7 graphically present the 2010 Build weekday morning and evening peak-hour traffic volume networks.

Figure 6: 2010 Build
Weekday Morning
Peak Hour Traffic Volumes

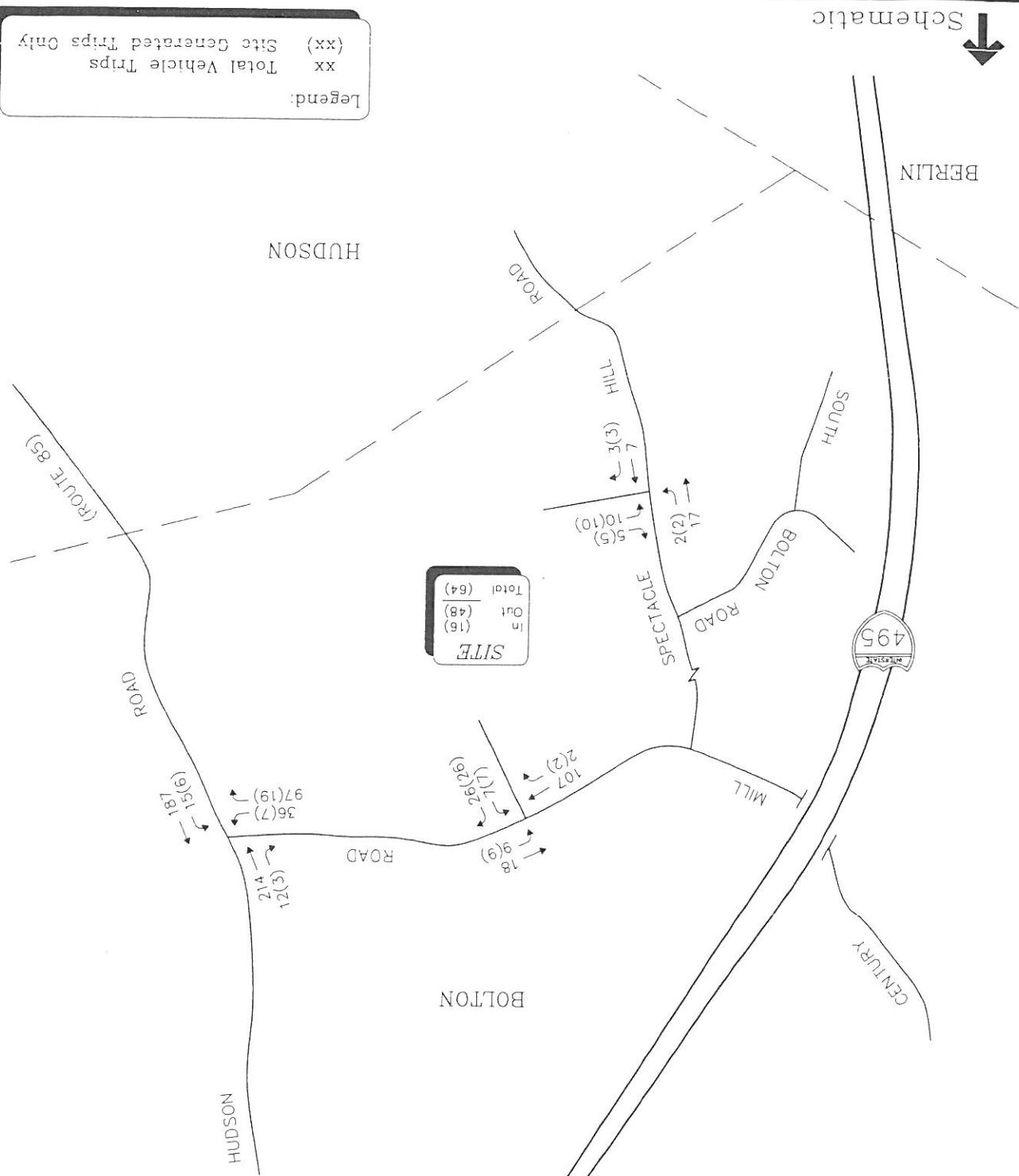
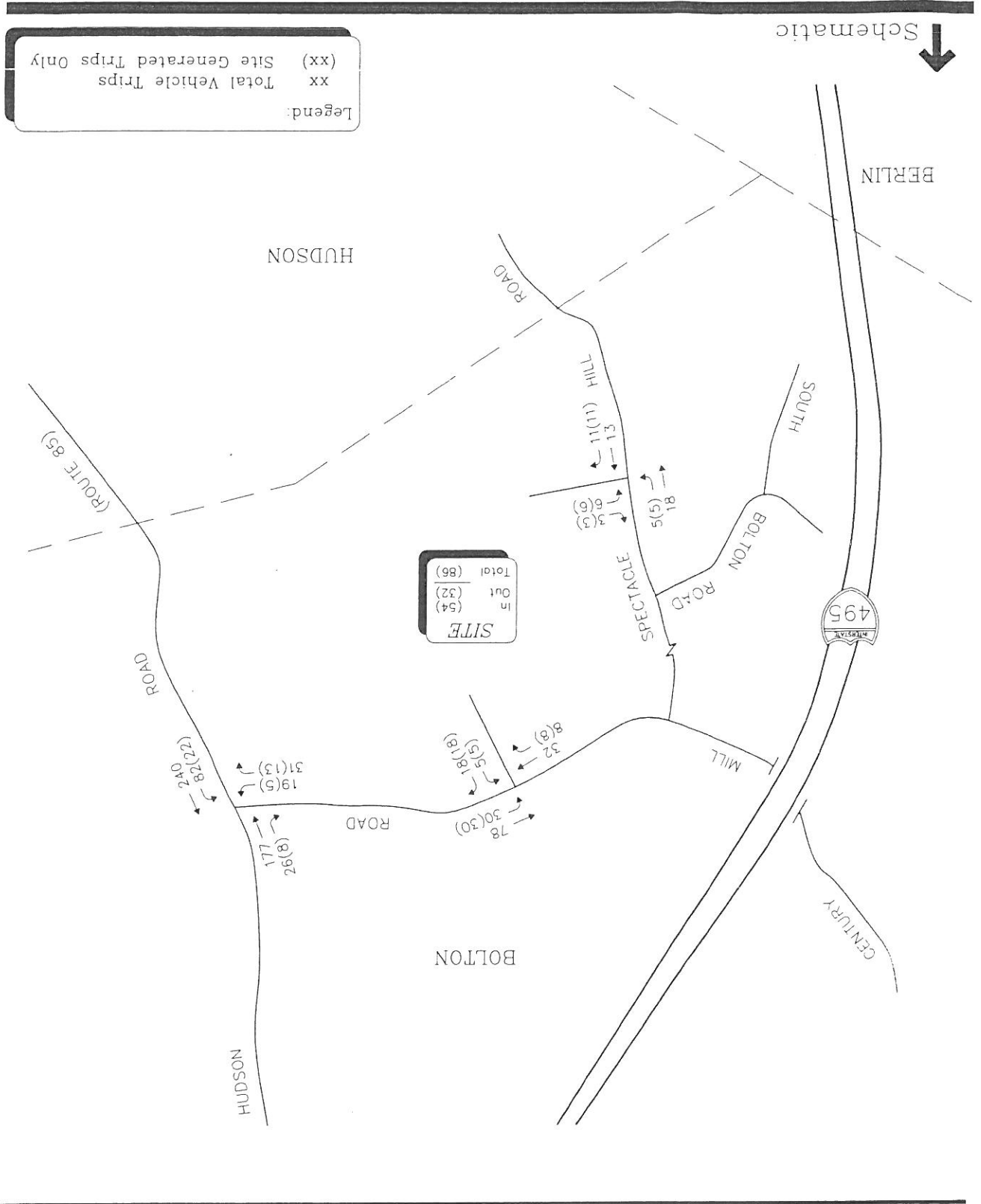


TABLE 4
PROJECT-GENERATED VEHICLE-TRIP ASSIGNMENT

Time Period/ Direction of Travel	Site Driveway	Via Century Mill Road	Via West Century Mill Road	Via North Spectacle Hill Road	Via South Spectacle Hill Road						
Weekday Morning	Peak Hour	Entering (vph) ^a	16	9	2	2	7	5	2	3	10
	Peak Hour	Exiting (vph)	48	26	9	7	5	7	2	10	13
	Weekday Evening	Total (vph)	64	35	9	7	7	7	2	13	13
Peak Hour	Peak Hour	Entering (vph)	54	30	8	8	5	5	5	11	6
	Peak Hour	Exiting (vph)	32	18	5	5	3	3	8	6	17
	Peak Hour	Total (vph)	86	48	13	13	8	8	13	17	17
Average Weekday	Average Weekday	Entering (vph) ^b	414	228	62	62	41	41	41	83	83
	Average Weekday	Exiting (vph)	414	228	62	62	41	41	41	83	83
	Average Weekday	Total (vph)	828	456	124	124	82	82	82	166	166

^avph = vehicle-trips per hour.
^bvph = vehicle-trips per day.

Figure 7: 2010 Build
 Weekday Evening
 Peak Hour Traffic Volumes



TRAFFIC ANALYSIS

To assess quality of flow along the study area roadways and intersections, an Intersection Level of Service (LOS) Analysis was conducted under the following analysis conditions:

- 2005 Existing Traffic Volume Conditions
- 2010 No Build Traffic Volume Conditions
- 2010 Build Traffic Volume Conditions

The LOS Analysis will provide an indication of how well the roadway facilities serve the traffic demands placed upon them.

METHODOLOGY

Level of Service

The primary result of capacity analysis is the assignment of level of service to traffic facilities under various traffic-flow conditions.¹ The concept of level of service is defined as a qualitative measure describing operational conditions within a traffic stream and their perception by motorists and/or passengers. A level-of-service definition provides an index to quality of traffic flow in terms of such factors as speed, travel time, freedom to maneuver, traffic interruptions, comfort, safety, and convenience.

Six levels of service are defined for each type of facility. They are given letter designations from A to F, with LOS A representing the best operating conditions and LOS F the worst.

¹Highway Capacity Manual (HCM 2000): Transportation Research Board, Washington, DC, 2000.

Since the level of service of a traffic facility is a function of the traffic flows placed upon it, such a facility may operate at a wide range of levels of service, depending on the time of day, day of week, or period of year.

Unsignalized Intersections

The levels of service for unsignalized intersections are determined by application of a procedure described in the HCM 2000 *Highway Capacity Manual*.² The procedure accounts for lane configuration on both the minor and major approaches, conflicting traffic stream volumes, and type of intersection control (STOP versus YIELD). First, theoretical maximum or capacity flow of vehicles for each minor approach lane is calculated. The capacities are then compared to the demand at the respective minor approaches. The delay is then calculated based on the critical gap time, move-up time as well as other various factors. Table 5 summarizes the relationship between level of service and expected delay to minor street traffic.

TABLE 5
LEVEL-OF-SERVICE CRITERIA FOR STOP-CONTROLLED INTERSECTIONS^a

Level of Service	Average Control Delay (Seconds/Vehicle)
A	≤ 10
B	> 10 - 15
C	> 15 - 25
D	> 25 - 35
E	> 35 - 50
F	> 50

^aSource: *Highway Capacity Manual*, (HCM 2000); Transportation Research Board, Washington, DC, 2000.

TRAFFIC ANALYSIS RESULTS

Capacity analyses have been conducted at the study area intersections. Results of these analyses are summarized below by intersection and are tabulated in Table 6.

Century Mill Road at the Proposed North Site Drive

Under 2010 Build with development traffic volume conditions, the left- and right-turns exiting the site and the left-turns entering the site would operate at LOS A during both the morning and evening peak hours.

²*Highway Capacity Manual*, (HCM 2000); Transportation Research Board, Washington, DC, 2000.

Spectacle Hill Road at the Proposed West Site Drive

Similar to the Proposed North Site Drive intersection, the left-turns entering the site and the left- and right-turns exiting the site would operate at LOS A during both the morning and evening peak hours under 2010 Build with development traffic volume conditions.

Hudson Road at Century Mill Road

The left-turns movement on Hudson Road will operate at LOS A up through the 2010 Build traffic volume conditions. The left- and right-turns on Century Mill Road will operate at LOS B during both the morning and evening peak hours up through 2010 Build with development traffic volume conditions.

**TABLE 6
UNSIGNALIZED INTERSECTION LEVEL-OF-SERVICE SUMMARY**

Location/Peak Hour/ Turning Movement	2005 Existing			2010 No-Build			2010 Build		
	LOS ^a	ACD ^b	Demand ^c	LOS	ACD	Demand	LOS	ACD	Demand
Century Mill Road at the Proposed North Site Drive <i>Morning Peak Hour</i> Westbound Left-Turns Entering Northbound All Movements Exiting	-	---	--	-	---	--	A	7.6	9
Westbound Left-Turns Entering Northbound All Movements Exiting	-	---	--	-	---	--	A	9.5	35
<i>Evening Peak Hour</i> Westbound Left-Turns Entering Northbound All Movements Exiting	-	---	--	-	---	--	A	7.3	32
Exiting	-	---	--	-	---	--	A	8.8	24
Spectacle Hill Road at the Proposed West Site Drive <i>Morning Peak Hour</i> Southbound Left-Turns Entering Westbound All Move- ments Exiting	-	---	--	-	---	--	A	7.2	2
Westbound All Move- ments Exiting	-	---	--	-	---	--	A	8.6	15
<i>Evening Peak Hour</i> Southbound Left-Turns Entering Westbound All Move- ments Exiting	-	---	--	-	---	--	A	7.3	5
Westbound All Move- ments Exiting	-	---	--	-	---	--	A	8.7	9
Century Mill Road at Hudson Road <i>Morning Peak Hour</i> Northbound Left-Turns Eastbound Left & Right Turns	A	7.6	8	A	7.7	9	A	7.7	16
Right Turns	B	10.5	105	B	10.9	115	B	11.3	144
<i>Evening Peak Hour</i> Northbound Left-Turns Westbound Left & Right Turns	A	7.7	58	A	7.8	65	A	7.8	89
Right Turns	B	10.7	31	B	11.3	34	B	11.5	53

^aLevel of service.

^bAverage Control Delay (seconds/vehicle).

^cDemand approach volumes expressed in vehicles per hour.

SIGHT DISTANCE

Century Mill Road approaches the Proposed North Site Drive over a general level/tangent alignment with a large radius curve located to the east. The roadway alignment is generally tangent in the immediate vicinity of the Proposed West Site Drive. To the south of the Proposed West Site Drive, Spectacle Hill Road slopes down in the northbound direction at a rate of approximately 6% with a large radius curve located to the south. To the north, Spectacle Hill Road is generally level. Detailed Sight Distance measurements were conducted with respect to the Century Mill Road and Spectacle Hill Road approaches to the proposed new intersections to identify any potential mitigation measures, which would enhance the available Sight Distance.

Sight Distance considerations are basically divided into two criteria: (1) Stopping Sight Distance (SSD) and (2) Intersection Sight Distance (ISD).

STOPPING SIGHT DISTANCE (SSD)

SSD is the distance required for an approaching vehicle to perceive and react accordingly to an exiting or entering vehicle. The values are based upon a perception and reaction time of 2.5 seconds and a braking distance calculated for wet level pavement. When the roadway is either on an upgrade or downgrade greater than 3 percent, grade correction factors are applied.

INTERSECTION SIGHT DISTANCE (ISD)

ISD is based upon a perception and reaction time and the time required to complete a desired exiting maneuver after the decision to do so has been made. Values for ISD represent the time required to: (1) turn left or right and to accelerate to the operating speed of the street without causing approaching vehicles to reduce speed by more than 70% of the initial/design speed and (2) upon turning left, or crossing the major roadway, to clear the roadway without causing approaching vehicles to reduce speed by more than 70% of the initial/design speed.

SSD is far more important as it represents the minimum distance required for stopping, while the ISD criteria is based only upon acceptable speed reductions to the approaching traffic stream.

Table 7 compares the measured SSD with the minimum SSD requirements for both the 85th percentile observed vehicle speeds as well as the official speed limit along both Century Mill Road and Spectacle Hill Road in the vicinity of the proposed new intersections. As can be seen in Table 7 the available SSD along both Century Mill Road and Spectacle Hill Road exceeds the minimum SSD requirements for the official speed limit of 30 mph as well as the observed 85th percentile speed of up to 44 mph.

TABLE 7
SUMMARY OF STOPPING SIGHT DISTANCE ANALYSIS

Location/ Direction of Travel	Official Speed Limit ^a (mph)	85 th Percentile Observed Speed (mph)	Measured SSD ^b (feet)	SSD Requirements (feet) for Design Speed Condition ^c (mph)
Century Mill Road at the Proposed North Site Drive Intersection:				
East of the North Site Drive Vehicle B Approaching Looking West Vehicle A Exiting	40	43 WB	+338	305
Looking East	40	43 WB	+338	338
West of the North Site Drive Vehicle A Exiting Looking West Vehicle B Approaching	40	44 EB	+349	305
Looking East	40	44 EB	+349	349
Spectacle Hill Road at the Proposed West Site Drive Intersection:				
North of the West Site Drive Vehicle B Approaching Looking South Vehicle A Exiting	40	39 SB	+305	294
Looking North	40	39 SB	+305	305
South of the West Site Drive ^d Vehicle A Exiting Looking South Vehicle B Approaching	40	35 NB	+305	271
Looking North	40	35 NB	+305	305

^aMiles per hour.
^bSSD measurements occurred on December 22, 2003 and assumes the recommendations contained in this report are implemented.
^cSource: A Policy on Geometric Design of Highways and Streets, American Association of State Highway and Transportation Officials (AASHTO), 4th Edition, 2001.
^dSSD was increased to account for a downgrade of 6.0%. The existing downgrade ranged between 5.5 to 6.0%.

APPENDIX

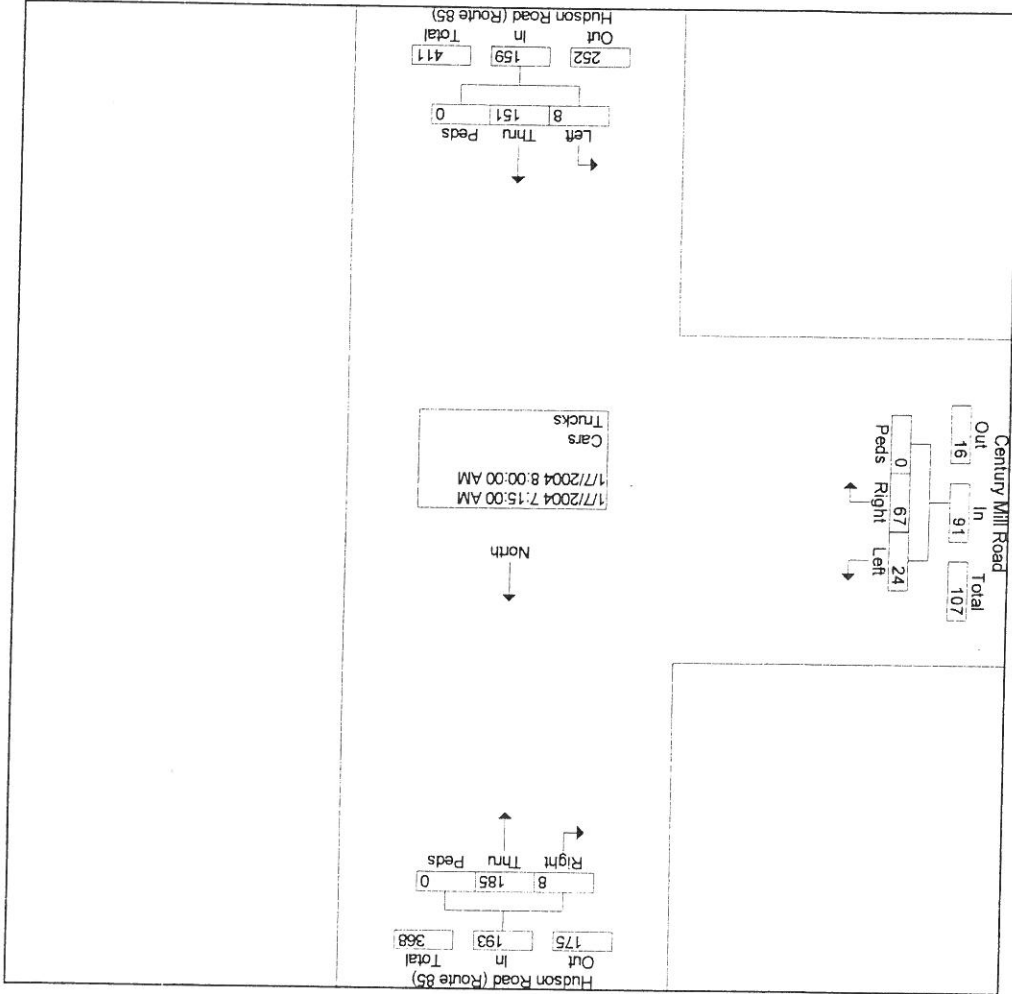
- TRAFFIC VOLUME AND VEHICLE SPEED DATA
- CAPACITY ANALYSIS

657-Rp13

TRAFFIC VOLUME AND VEHICLE SPEED DATA

N/S: Hudson Road (Route 85)
 W: Century Mill Road
 City, State: Bolton, MA
 Client: DJK/D. Kelly

Hudson Road (Route 85)				Hudson Road (Route 85)				Century Mill Road			
From North				From South				From West			
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Intersection 07:15 AM											
Volume	8	185	0	0	193	0	151	67	0	91	443
Percent	4.1	95.9	0.0	0.0	50	0.0	95.0	73.6	0.0	22	119
Peak Factor	1	49	0	0	50	0	46	19	0	0.931	
High Int. Volume	3	48	0	0	51	0	46	17	0	25	
Peak Factor					0.946					0.910	
Peak Hour From 07:00 AM to 08:45 AM - Peak 1 of 1											
07:30 AM											
Volume	1	49	0	0	50	0	46	19	0	22	119
Percent	4.1	95.9	0.0	0.0	50	0.0	95.0	73.6	0.0	22	119
Peak Factor	1	49	0	0	50	0	46	19	0	0.931	
High Int. Volume	3	48	0	0	51	0	46	17	0	25	
Peak Factor					0.946					0.910	
08:00 AM											
Volume	8	185	0	0	193	0	151	67	0	91	443
Percent	4.1	95.9	0.0	0.0	50	0.0	95.0	73.6	0.0	22	119
Peak Factor	1	49	0	0	50	0	46	19	0	0.931	
High Int. Volume	3	48	0	0	51	0	46	17	0	25	
Peak Factor					0.946					0.910	



Hudson Road (Route 85)
 Century Mill Road
 State: Bolton, MA
 DJK/D. Kelly

File Name : 02797A
 Site Code : 00000657
 Start Date : 01/07/2004
 Page No : 1

Hudson Road (Route 85)											
Groups Printed - Cars - Trucks											
Hudson Road (Route 85)						Century Mill Road					
From North						From West					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
07:00 AM	1	39	0	0	0	17	0	5	0	0	97
07:15 AM	2	45	0	0	0	15	0	7	0	0	115
07:30 AM	1	49	0	0	0	19	0	3	0	0	119
07:45 AM	2	43	0	0	0	17	0	8	0	0	109
Total	6	176	0	0	0	68	0	23	0	0	440
08:00 AM	3	48	0	0	0	16	0	6	0	0	100
08:15 AM	1	43	0	0	0	8	0	8	0	0	108
08:30 AM	3	24	0	0	0	7	0	4	0	0	72
08:45 AM	4	38	0	0	0	8	0	4	0	0	82
Total	11	153	0	0	0	39	0	22	0	0	362
Grand Total	17	329	0	0	0	107	0	45	0	0	802
Approch %	4.9	95.1	0.0	0.0	0.0	70.4	0.0	29.6	0.0	0.0	
Total %	2.1	41.0	0.0	0.0	0.0	13.3	0.0	5.6	0.0	0.0	
Hudson Road (Route 85)											
From South											
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
07:00 AM	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0
08:00 AM	0	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	0	0	0	0	0	0	0	0	0	0
08:30 AM	0	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	0	0	0	0	0	0	0	0	0	0
Approch %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Century Mill Road											
From West											
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
07:00 AM	0	0	0	0	0	0	0	0	0	0	0
07:15 AM	0	0	0	0	0	0	0	0	0	0	0
07:30 AM	0	0	0	0	0	0	0	0	0	0	0
07:45 AM	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0
08:00 AM	0	0	0	0	0	0	0	0	0	0	0
08:15 AM	0	0	0	0	0	0	0	0	0	0	0
08:30 AM	0	0	0	0	0	0	0	0	0	0	0
08:45 AM	0	0	0	0	0	0	0	0	0	0	0
Total	0	0	0	0	0	0	0	0	0	0	0
Grand Total	0	0	0	0	0	0	0	0	0	0	0
Approch %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	
Total %	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	

Hudson Road (Route 85)											
From North											
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
r From 07:00 AM to 08:45 AM - Peak 1 of 1											
07:15 AM	8	185	0	0	193	0	151	8	0	159	443
Volume	4.1	95.9	0.0	0.0	50	0.0	95.0	5.0	0.0	47	
Percent	1	49	0	0	46	0	46	1	0	47	
< Factor	3	48	0	0	51	0	46	1	0	47	
High Int. Volume	07:30 AM										
< Factor	0.946										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.846										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.910										
Volume	0	0	0	0	0	0	0	0	0	0	
< Factor	0.931										

Hudson Road (Route 85)

Century Mill Road

State: Bolton, MA

: DJK/D. Kelly

File Name : 02797A
Site Code : 00000657
Start Date : 01/07/2004
Page No : 1

Hudson Road (Route 85)											
From North						From South					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
07:00 AM	1	37	0	0	0	2	32	0	0	0	94
07:15 AM	2	43	0	0	0	3	41	0	0	0	111
07:30 AM	1	48	0	0	0	1	44	0	0	0	116
07:45 AM	2	42	0	0	0	2	36	0	0	0	107
Total	6	170	0	0	0	8	153	0	0	0	428
08:00 AM	3	47	0	0	0	1	23	0	0	0	96
08:15 AM	1	43	0	0	0	2	45	0	0	0	107
08:30 AM	3	21	0	0	0	1	31	0	0	0	67
08:45 AM	4	38	0	0	0	4	24	0	0	0	82
Total	11	149	0	0	0	8	123	0	0	0	352
Grand Total	17	319	0	0	0	16	276	0	0	0	780
Approch %	5.1	94.9	0.0	0.0	0.0	5.5	94.5	0.0	0.0	0.0	0.0
Total %	2.2	40.9	0.0	0.0	0.0	2.1	35.4	0.0	0.0	0.0	0.0
Century Mill Road											
From West						From East					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
07:00 AM	17	68	0	0	0	107	45	0	0	0	25
07:15 AM	15	17	0	0	0	73.6	19	0	0	0	22
07:30 AM	19	17	0	0	0	26.4	3	0	0	0	116
07:45 AM	8	0	0	0	0	26.4	3	0	0	0	116
Total	47	103	0	0	0	151	67	0	0	0	430
Approch %	13.7	70.4	0.0	0.0	0.0	45	73.6	0.0	0.0	0.0	0.927
Total %	0.0	13.7	0.0	0.0	0.0	45	73.6	0.0	0.0	0.0	0.910

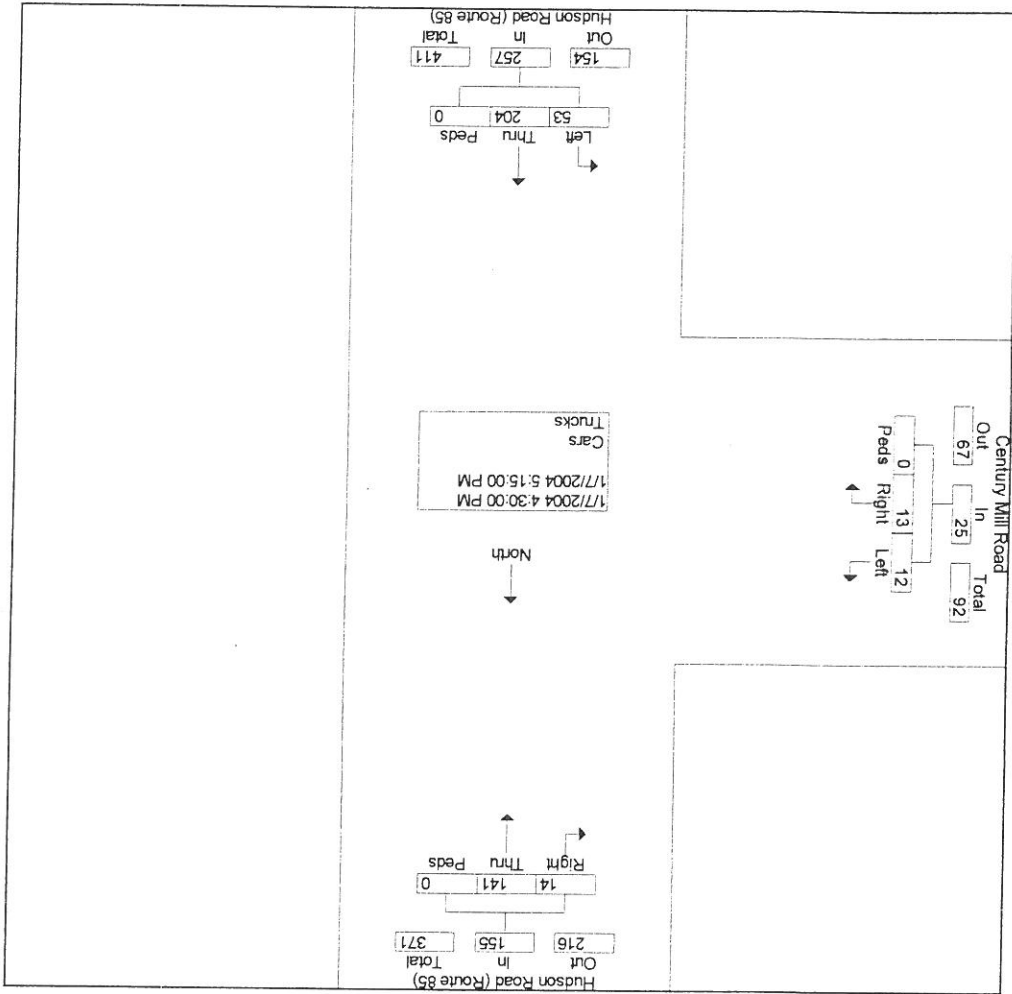
1/S: Hudson Road (Route 85)
 V: Century Mill Road
 City, State: Bolton, MA
 Client: DJK/D. Kelly

Transportation Data Corporation
 P.O. Box 734 Natick, MA 01760
 Office: 508-651-1610 Fax: 508-651-1229

File Name : 02797AA
 Site Code : 00000657
 Start Date : 01/07/2004
 Page No : 1

Hudson Road (Route 85)									
From North					From South				
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds
Intersection 04:30 PM	14	141	0	0	155	0	204	53	0
Volume	14	141	0	0	155	0	204	53	0
Percent	9.0	91.0	0.0	0.0	38	0.0	79.4	20.6	0.0
Peak Factor	3	35	0	0	43	0	53	17	0
High Int. 04:30 PM	7	36	0	0	43	0	53	17	0
Peak Factor	0.901	0.901	0	0	0.901	0.901	0.901	0.901	0.901

Century Mill Road									
From West					From East				
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds
Intersection 04:30 PM	14	141	0	0	155	0	204	53	0
Volume	14	141	0	0	155	0	204	53	0
Percent	9.0	91.0	0.0	0.0	38	0.0	79.4	20.6	0.0
Peak Factor	3	35	0	0	43	0	53	17	0
High Int. 04:30 PM	7	36	0	0	43	0	53	17	0
Peak Factor	0.901	0.901	0	0	0.901	0.901	0.901	0.901	0.901



Hudson Road (Route 85)
 Century Mill Road
 State: Bolton, MA
 DJK/D. Kelly

File Name : 02797AA
 Site Code : 00000657
 Start Date : 01/07/2004
 Page No : 1

Hudson Road (Route 85)											
From North						From South					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
04:00 PM	2	34	39	0	0	9	43	38	0	0	89
04:15 PM	6	39	36	0	0	8	38	46	0	0	95
04:30 PM	7	36	31	0	0	11	49	46	0	0	108
04:45 PM	2	31	0	0	0	10	49	0	0	0	96
Total	17	140	0	0	0	38	176	0	0	0	388
05:00 PM	2	39	0	0	0	15	54	0	0	0	115
05:15 PM	3	35	0	0	0	17	53	0	0	0	115
05:30 PM	6	30	0	0	0	17	39	0	0	0	101
05:45 PM	7	45	0	0	0	7	35	0	0	0	101
Total	18	149	0	0	0	56	181	0	0	0	432
Grand Total	35	289	0	0	0	94	357	0	0	0	820
Approach %	10.8	89.2	0.0	0.0	0.0	20.8	79.2	0.0	0.0	0.0	0.0
Total %	4.3	35.2	0.0	0.0	0.0	11.5	43.5	0.0	0.0	0.0	0.0
Hudson Road (Route 85)											
From North						From South					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
04:00 PM	14	141	0	0	155	0	202	53	0	255	434
04:15 PM	9.0	91.0	0.0	0.0	38	0.0	79.2	20.8	0.0	70	115
04:30 PM	3	35	0	0	38	0	53	17	0	70	115
04:45 PM	7	36	0	0	43	0	53	17	0	70	115
Total	36	289	0	0	201	0	357	94	0	451	820
Approach %	10.8	89.2	0.0	0.0	0.0	20.8	79.2	0.0	0.0	0.0	0.0
Total %	4.3	35.2	0.0	0.0	0.0	11.5	43.5	0.0	0.0	0.0	0.0
Century Mill Road											
From West						From East					
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
04:00 PM	1	3	0	0	4	1	9	0	0	10	89
04:15 PM	1	4	0	0	5	1	8	0	0	9	95
04:30 PM	2	3	0	0	5	2	7	0	0	9	101
04:45 PM	2	3	0	0	5	2	7	0	0	9	101
Total	6	10	0	0	16	6	23	0	0	29	388
Approach %	10.8	89.2	0.0	0.0	0.0	20.8	79.2	0.0	0.0	0.0	0.0
Total %	4.3	35.2	0.0	0.0	0.0	11.5	43.5	0.0	0.0	0.0	0.0

[illegible]

Century Mill Road
West of Site Driveway
City, State: Bolton, MA
Client: D/K/D. Kelly

Site Code: 65
02797Bvolturn

Start Time	EB	WB	Combined	Peak	Volume	H.F.
12:00	1	0	1	07:15	0.875	91
12:15	2	0	2	05:00	0.700	28
12:30	4	0	4	08:15	0.708	17
12:45	7	1	8	05:00	0.784	69
01:00	2	2	4	07:15	0.867	104
01:15	0	1	1	05:00	0.866	97
01:30	0	1	1			
01:45	0	2	2			
02:00	0	2	2			
02:15	0	2	2			
02:30	0	2	2			
02:45	0	2	2			
03:00	0	2	2			
03:15	0	2	2			
03:30	0	2	2			
03:45	0	2	2			
04:00	0	2	2			
04:15	0	2	2			
04:30	1	2	3			
04:45	2	3	5			
05:00	1	3	4			
05:15	0	3	3			
05:30	1	3	4			
05:45	0	3	3			
06:00	0	3	3			
06:15	2	3	5			
06:30	13	3	16			
06:45	14	3	17			
07:00	16	3	19			
07:15	26	3	29			
07:30	19	3	22			
07:45	26	3	29			
08:00	20	3	23			
08:15	14	3	17			
08:30	12	3	15			
08:45	8	3	11			
09:00	9	3	12			
09:15	3	3	6			
09:30	1	3	4			
09:45	6	3	9			
10:00	4	3	7			
10:15	6	3	9			
10:30	1	3	4			
10:45	5	3	8			
11:00	1	3	4			
11:15	2	3	5			
11:30	8	3	11			
11:45	2	3	5			
Total	238	74	312			
Percent	76.3%	23.7%	67.1%			
Day Total	398	401	799			

Start Time	EB	WB	Combined	Peak Volume	P.H.F.
12:00	0	0	0	07:15	0.848
12:15	0	0	0	07:15	0.848
12:30	0	0	0	07:15	0.848
12:45	0	0	0	07:15	0.848
01:00	0	0	0	07:15	0.848
01:15	0	0	0	07:15	0.848
01:30	0	0	0	07:15	0.848
01:45	0	0	0	07:15	0.848
02:00	0	0	0	07:15	0.848
02:15	0	0	0	07:15	0.848
02:30	0	0	0	07:15	0.848
02:45	0	0	0	07:15	0.848
03:00	0	0	0	07:15	0.848
03:15	0	0	0	07:15	0.848
03:30	0	0	0	07:15	0.848
03:45	0	0	0	07:15	0.848
04:00	0	0	0	07:15	0.848
04:15	0	0	0	07:15	0.848
04:30	0	0	0	07:15	0.848
04:45	1	1	2	07:15	0.848
05:00	0	0	0	07:15	0.848
05:15	0	0	0	07:15	0.848
05:30	0	0	0	07:15	0.848
05:45	3	0	3	07:15	0.848
06:00	2	0	2	07:15	0.848
06:15	4	0	4	07:15	0.848
06:30	9	0	9	07:15	0.848
06:45	18	0	18	07:15	0.848
07:00	13	0	13	07:15	0.848
07:15	22	0	22	07:15	0.848
07:30	21	0	21	07:15	0.848
07:45	28	0	28	07:15	0.848
08:00	24	0	24	07:15	0.848
08:15	13	0	13	07:15	0.848
08:30	16	0	16	07:15	0.848
08:45	9	0	9	07:15	0.848
09:00	5	0	5	07:15	0.848
09:15	8	0	8	07:15	0.848
09:30	6	0	6	07:15	0.848
09:45	3	0	3	07:15	0.848
10:00	6	0	6	07:15	0.848
10:15	7	0	7	07:15	0.848
10:30	6	0	6	07:15	0.848
10:45	3	0	3	07:15	0.848
11:00	2	0	2	07:15	0.848
11:15	3	0	3	07:15	0.848
11:30	5	0	5	07:15	0.848
11:45	5	0	5	07:15	0.848
Total	244	79	323		
Percent	75.5%	24.5%	71.1%		
Day Total	363	372	735		

Time	Start	End	Day	Total	Percent	Peak	P.H.F.	Volume
12:00	12:15	12:30	126	42	44.7%	09:45	0.600	12
12:15	12:30	12:45	126	4	50.0%	00:15	0.594	19
12:30	12:45	12:55	126	0	55.3%	07:45	0.875	14
12:45	12:55	13:00	136	2	50.0%	05:00	0.625	15
12:55	13:00	13:05	136	6	50.0%	08:30	0.719	23
13:00	13:05	13:10	136	0	50.0%	00:15	0.675	27
13:05	13:10	13:15	136	0	50.0%			
13:10	13:15	13:15	136	0	50.0%			
13:15	13:15	13:20	136	0	50.0%			
13:20	13:20	13:25	136	0	50.0%			
13:25	13:25	13:30	136	0	50.0%			
13:30	13:30	13:35	136	0	50.0%			
13:35	13:35	13:40	136	0	50.0%			
13:40	13:40	13:45	136	0	50.0%			
13:45	13:45	13:50	136	0	50.0%			
13:50	13:50	13:55	136	0	50.0%			
13:55	13:55	14:00	136	0	50.0%			
14:00	14:00	14:05	136	0	50.0%			
14:05	14:05	14:10	136	0	50.0%			
14:10	14:10	14:15	136	0	50.0%			
14:15	14:15	14:20	136	0	50.0%			
14:20	14:20	14:25	136	0	50.0%			
14:25	14:25	14:30	136	0	50.0%			
14:30	14:30	14:35	136	0	50.0%			
14:35	14:35	14:40	136	0	50.0%			
14:40	14:40	14:45	136	0	50.0%			
14:45	14:45	14:50	136	0	50.0%			
14:50	14:50	14:55	136	0	50.0%			
14:55	14:55	15:00	136	0	50.0%			
15:00	15:00	15:05	136	0	50.0%			
15:05	15:05	15:10	136	0	50.0%			
15:10	15:10	15:15	136	0	50.0%			
15:15	15:15	15:20	136	0	50.0%			
15:20	15:20	15:25	136	0	50.0%			
15:25	15:25	15:30	136	0	50.0%			
15:30	15:30	15:35	136	0	50.0%			
15:35	15:35	15:40	136	0	50.0%			
15:40	15:40	15:45	136	0	50.0%			
15:45	15:45	15:50	136	0	50.0%			
15:50	15:50	15:55	136	0	50.0%			
15:55	15:55	16:00	136	0	50.0%			
16:00	16:00	16:05	136	0	50.0%			
16:05	16:05	16:10	136	0	50.0%			
16:10	16:10	16:15	136	0	50.0%			
16:15	16:15	16:20	136	0	50.0%			
16:20	16:20	16:25	136	0	50.0%			
16:25	16:25	16:30	136	0	50.0%			
16:30	16:30	16:35	136	0	50.0%			
16:35	16:35	16:40	136	0	50.0%			
16:40	16:40	16:45	136	0	50.0%			
16:45	16:45	16:50	136	0	50.0%			
16:50	16:50	16:55	136	0	50.0%			
16:55	16:55	17:00	136	0	50.0%			
17:00	17:00	17:05	136	0	50.0%			
17:05	17:05	17:10	136	0	50.0%			
17:10	17:10	17:15	136	0	50.0%			
17:15	17:15	17:20	136	0	50.0%			
17:20	17:20	17:25	136	0	50.0%			
17:25	17:25	17:30	136	0	50.0%			
17:30	17:30	17:35	136	0	50.0%			
17:35	17:35	17:40	136	0	50.0%			
17:40	17:40	17:45	136	0	50.0%			
17:45	17:45	17:50	136	0	50.0%			
17:50	17:50	17:55	136	0	50.0%			
17:55	17:55	18:00	136	0	50.0%			
18:00	18:00	18:05	136	0	50.0%			
18:05	18:05	18:10	136	0	50.0%			
18:10	18:10	18:15	136	0	50.0%			
18:15	18:15	18:20	136	0	50.0%			
18:20	18:20	18:25	136	0	50.0%			
18:25	18:25	18:30	136	0	50.0%			
18:30	18:30	18:35	136	0	50.0%			
18:35	18:35	18:40	136	0	50.0%			
18:40	18:40	18:45	136	0	50.0%			
18:45	18:45	18:50	136	0	50.0%			
18:50	18:50	18:55	136	0	50.0%			
18:55	18:55	19:00	136	0	50.0%			
19:00	19:00	19:05	136	0	50.0%			
19:05	19:05	19:10	136	0	50.0%			
19:10	19:10	19:15	136	0	50.0%			
19:15	19:15	19:20	136	0	50.0%			
19:20	19:20	19:25	136	0	50.0%			
19:25	19:25	19:30	136	0	50.0%			
19:30	19:30	19:35	136	0	50.0%			
19:35	19:35	19:40	136	0	50.0%			
19:40	19:40	19:45	136	0	50.0%			
19:45	19:45	19:50	136	0	50.0%			
19:50	19:50	19:55	136	0	50.0%			
19:55	19:55	20:00	136	0	50.0%			
20:00	20:00	20:05	136	0	50.0%			
20:05	20:05	20:10	136	0	50.0%			
20:10	20:10	20:15	136	0	50.0%			
20:15	20:15	20:20	136	0	50.0%			
20:20	20:20	20:25	136	0	50.0%			
20:25	20:25	20:30	136	0	50.0%			
20:30	20:30	20:35	136	0	50.0%			
20:35	20:35	20:40	136	0	50.0%			
20:40	20:40	20:45	136	0	50.0%			
20:45	20:45	20:50	136	0	50.0%			
20:50	20:50	20:55	136	0	50.0%			
20:55	20:55	21:00	136	0	50.0%			
21:00	21:00	21:05	136	0	50.0%			
21:05	21:05	21:10	136	0	50.0%			
21:10	21:10	21:15	136	0	50.0%			
21:15	21:15	21:20	136	0	50.0%			
21:20	21:20	21:25	136	0	50.0%			
21:25	21:25	21:30	136	0	50.0%			
21:30	21:30	21:35	136	0	50.0%			
21:35	21:35	21:40	136	0	50.0%			
21:40	21:40	21:45	136	0	50.0%			
21:45	21:45	21:50	136	0	50.0%			
21:50	21:50	21:55	136	0	50.0%			
21:55	21:55	22:00	136	0	50.0%			
22:00	22:00	22:05	136	0	50.0%			
22:05	22:05	22:10	136	0	50.0%			
22:10	22:10	22:15	136	0	50.0%			
22:15	22:15	22:20	136	0	50.0%			
22:20	22:20	22:25	136	0	50.0%			
22:25	22:25	22:30	136	0	50.0%			
22:30	22:30	22:35	136	0	50.0%			
22:35	22:35	22:40	136	0	50.0%			
22:40	22:40	22:45	136	0	50.0%			
22:45	22:45	22:50	136	0	50.0%			
22:50	22:50	22:55	136	0	50.0%			
22:55	22:55	23:00	136	0	50.0%			
23:00	23:00	23:05	136	0	50.0%			
23:05	23:05	23:10	136	0	50.0%			
23:10	23:10	23:15	136	0	50.0%			
23:15	23:15	23:20	136	0	50.0%			
23:20	23:20	23:25	136	0	50.0%			
23:25	23:25	23:30	136	0	50.0%			
23:30	23:30	23:35	136	0	50.0%			
23:35	23:35	23:40	136	0	50.0%			
23:40	23:40	23:45	136	0	50.0%			
23:45	23:45	23:50	136	0	50.0%			
23:50	23:50	23:55	136	0	50.0%			
23:55	23:55	24:00	136	0	50.0%			
24:00	24:00	24:05	136	0	50.0%			
24:05	24:05	24:10	136	0	50.0%			
24:10	24:10	24:15	136	0	50.0%			
24:15	24:15	24:20	136	0	50.0%			
24:20	24:20	24:25	136	0	50.0%			
24:25	24:25	24:30	136	0	50.0%			
24:30	24:30	24:35	136	0	50.0%			
24:35	24:35	24:40	136	0	50.0%			
24:40	24:40	24:45	136	0	50.0%			
24:45	24:45	24:50	136	0	50.0%			
24:50	24:50	24:55	136	0	50.0%			
24:55	24:55	25:00	136	0	50.0%			
25:00	25:00	25:05	136	0	50.0%			
25:05	25:05	25:10	136	0	50.0%			
25:10	25:10	25:15	136	0	50.0%			
25:15	25:15	25:20	136	0	50.0%			
25:20	25:20	25:25	136	0	50.0%			
25:25	25:25	25:30	136	0	50.0%			
25:30	25:30	25:35	136	0	50.0%			
25:35	25:35	25:40	136	0	50.0%			
25:40	25:40	25:45	136	0	50.0%			
25:45	25:45	25:50	136	0	50.0%			
25:50	25:50	25:55	136	0	50.0%			
25:55	25:55	26:00	136	0	50.0%			
26:00	26:00	26:05	136	0	50.0%			
26:05	26:05	26:10	136	0	50.0%			
26:10	26:10	26:15	136	0	50.0%			
26:15	26:15	26:20	136	0	50.0%			
26:20	26:20	26:25	136	0	50.0%			
26:25	26:25	26:30	136	0	50.0%			
26:30	26:30	26:35	136	0	50.0%			
26:35	26:35	26:40	136	0	50.0%			
26:40	26:40	26:45	136	0	50.0%			
26:45	26:45	26:50	136	0	50.0%			
26:50	26:50							

Century Mill Road
West of Site Driveway
City, State: Bolton, MA
Client: DJR/D. Kelly

TDC
Transportation Data Corporation
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Site Code: 657
02797BSPFED

Start Time	01/07/04	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
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15th Percentile: 30 MPH
50th Percentile: 36 MPH
85th Percentile: 42 MPH
95th Percentile: 44 MPH
10 MPH Pace Speed: 30-39 MPH
Number in Pace: 262
Percent in Pace: 65.8%
Number of Vehicles > 35 MPH: 212
Percent of Vehicles > 35 MPH: 53.3%
Mean Speed(Average): 36 MPH

%ile
Speed
Stats

Century Mill Road
West of Site Driveway
City, Sare: Bolton, MA
Client: DJR/D. Kelly

Office: 508-651-1610 Fax: 508-651-1229
P.O. Box 734 Nahick, MA 01760

TDC
Transportation Data Corporation

Site Code: 657
02797BSPFED

Start Time	1	15	19	24	29	34	39	44	49	54	59	64	69	70	Total	Ave. Speed
04:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:00	0	0	0	0	0	0	2	3	0	0	0	0	0	0	5	40
06:00	0	0	0	0	0	7	11	10	2	1	1	0	0	0	33	40
07:00	0	0	0	0	0	10	26	38	8	1	1	0	0	0	84	39
08:00	0	0	0	0	0	6	22	20	11	3	0	0	0	0	62	41
09:00	0	0	0	0	0	6	6	5	3	1	0	0	0	0	22	38
10:00	0	0	0	0	0	9	6	6	2	3	0	0	0	0	22	36
11:00	0	0	0	0	1	1	7	5	1	0	0	0	0	0	15	38
12 PM	0	0	0	1	2	2	5	6	2	0	0	0	0	0	18	37
13:00	1	0	0	0	0	4	3	0	0	0	0	0	0	0	10	32
14:00	0	0	0	0	0	4	5	6	2	0	0	0	0	0	18	37
15:00	0	0	0	0	0	0	7	6	1	0	0	0	0	0	11	37
16:00	0	0	0	0	0	8	8	3	0	0	0	0	0	0	19	36
17:00	0	0	1	0	0	3	3	0	0	0	0	0	0	0	15	32
18:00	0	0	1	0	0	5	1	0	0	0	0	0	0	0	9	28
19:00	0	0	2	1	0	1	4	0	0	0	0	0	0	0	9	28
20:00	0	0	0	0	0	3	1	0	0	0	0	0	0	0	6	31
21:00	1	0	0	0	0	2	1	0	0	0	0	0	0	0	4	24
22:00	0	0	0	0	0	1	1	0	0	0	0	0	0	0	4	30
23:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	*
Total	3	4	2	20	68	120	106	31	7	2	0	0	0	0	363	*
Percent	0.8%	1.1%	0.6%	5.5%	18.7%	33.1%	29.2%	8.5%	1.9%	0.6%	0.0%	0.0%	0.0%	0.0%		

Speed
%ile
15th Percentile: 31 MPH
50th Percentile: 38 MPH
85th Percentile: 44 MPH
95th Percentile: 48 MPH
Stats
10 MPH Pace Speed: 35-44 MPH
Number in Pace: 226
Percent in Pace: 62.3%
Number of Vehicles > 35 MPH: 242
Percent of Vehicles > 35 MPH: 66.7%
Mean Speed(Average): 38 MPH

Start Time	01/07/	Speed																Stats
		1	14	19	24	29	34	39	44	49	54	59	64	69	70	Total		
40	2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
29	10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
37	13	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
37	9	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
37	16	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
37	21	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
35	21	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
32	15	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
39	25	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
37	41	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
38	51	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
38	69	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
38	46	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
39	23	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
36	10	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
34	13	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
37	12	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		
35	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0		

Site Code: 657
02797BSPFED

TDC
Transportation Data Corporation
P.O. Box 734 Natick, MA 01760
Office: 508-651-1610 Fax: 508-651-1229

Century Mill Road
West of Site Driveway
City, State: Bolton, MA
Client: DJK/D. Kelly
WB

Time	Start	WB	Speed														Stats
			15th Percentile	50th Percentile	85th Percentile	95th Percentile	31 MPH	37 MPH	43 MPH	46 MPH	35-44 MPH	227	61.0%	230	Percent of Vehicles > 35 MPH	Mean Speed(Average)	
01/08/04	15	14	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
02:00	20	19	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
03:00	25	24	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
04:00	30	29	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
05:00	35	34	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
06:00	40	39	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
07:00	45	44	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
08:00	50	49	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
09:00	55	54	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
10:00	60	65	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
11:00	65	69	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
12 PM	70	999	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
13:00	75	999	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
14:00	80	999	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
15:00	85	999	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
16:00	90	999	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
17:00	95	999	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
18:00	100	999	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
19:00	105	999	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
20:00	110	999	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
21:00	115	999	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
22:00	120	999	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
23:00	125	999	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
Total	130	97	26	2	1	0	0	0	0	0	0	0	0	0	0	0	
Percent	26.1%	7.0%	0.5%	0.3%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	0.0%	

Spectacle Hill Road
north of Site Driveway
City, State: Bolton, MA
Client: DJK/D. Kelly
NB

TDC
Transportation Data Corporation
P.O. Box 734 Natick, MA 01760
Office: 508-651-1610 Fax: 508-651-1229

Site Code: 657
02797ASPEED

Start Time	01/07/04	01:00	02:00	03:00	04:00	05:00	06:00	07:00	08:00	09:00	10:00	11:00	12 PM	13:00	14:00	15:00	16:00	17:00	18:00	19:00	20:00	21:00	22:00	23:00	Total	Percent	t
1	14	0	0	0	0	0	0	0	0	0	0	7	0	0	1	0	0	0	0	0	0	0	0	0	8	6.3%	6.3%
15	19	0	0	0	0	0	0	0	0	2	0	0	0	2	0	1	0	0	0	0	1	0	0	0	6	4.8%	4.8%
20	24	0	0	0	0	1	2	1	0	0	1	1	1	0	1	0	0	0	0	0	1	0	0	0	10	7.9%	7.9%
25	29	0	0	0	0	0	0	1	0	0	5	4	0	3	3	1	2	1	4	2	3	2	3	0	35	27.8%	27.8%
30	34	0	0	0	0	0	0	0	2	0	1	3	2	4	0	4	4	1	4	2	1	0	0	0	44	34.9%	34.9%
35	39	0	0	0	0	0	0	0	0	0	0	3	2	1	0	1	0	1	1	0	6	2	0	0	20	15.9%	15.9%
40	44	0	0	0	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	3	2.4%	2.4%
45	49	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0%	0.0%
50	54	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0%	0.0%
55	59	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0%	0.0%
60	64	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0%	0.0%
65	69	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0%	0.0%
70	999	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0%	0.0%
Total		1	0	0	0	0	0	0	0	0	0	10	9	10	4	15	12	9	8	6	7	5	4	1	126		
Ave Speed		30	0	0	0	0	0	0	0	0	0	26	29	24	30	30	18	29	28	31	32	28	34	23	30	*	

15th Percentile:	22 MPH
50th Percentile:	30 MPH
85th Percentile:	35 MPH
95th Percentile:	39 MPH
10 MPH Pace Speed:	25-34 MPH
Number in Pace:	79
Percent in Pace:	62.7%
Number of Vehicles > 30 MPH:	58
Percent of Vehicles > 30 MPH:	46.0%
Mean Speed(Average):	28 MPH

Spectacle Hill Road
north of Site Driveway
City, State: Bolton, MA
Client: DJK/D. Kelly
NB

TDC
Transportation Data Corporation
P.O. Box 734 Natick, MA 01760
Office: 508-651-1610 Fax: 508-651-1229

Site Code: 657
02797ASPEED

Start Time	01/08/	04	01:00	02:00	03:00	04:00	05:00	06:00	07:00	08:00	09:00	10:00	11:00	12 PM	13:00	14:00	15:00	16:00	17:00	18:00	19:00	20:00	21:00	22:00	23:00	Total	Percent	t
1	14	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0%	0
15	19	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	4.2%	4
20	24	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	18.8%	18
25	29	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	33.3%	32
30	34	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	26.0%	25
35	39	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	15.6%	15
40	44	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2.1%	2
45	49	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0%	0
50	54	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0%	0
55	59	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0%	0
60	64	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0%	0
65	69	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0%	0
70	999	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0%	0
Ave. Speed	Total	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	96	
		*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	*	

Speed
%ile
15th Percentile : 22 MPH
50th Percentile : 28 MPH
85th Percentile : 35 MPH
95th Percentile : 38 MPH
Stats
10 MPH Pace Speed : 25-34 MPH
Number in Pace : 57
Percent in Pace : 59.4%
Number of Vehicles > 30 MPH : 37
Percent of Vehicles > 30 MPH : 38.5%
Mean Speed(Average) : 29 MPH

Start Time	01/07/	04	01:00	02:00	03:00	04:00	05:00	06:00	07:00	08:00	09:00	10:00	11:00	12 PM	13:00	14:00	15:00	16:00	17:00	18:00	19:00	20:00	21:00	22:00	23:00	Total	Percent	1
1	14	15	19	20	24	25	29	30	34	35	39	40	44	45	49	50	54	55	59	60	64	65	69	70	999	Total	136	0
Ave Speed																												
15th Percentile: 23 MPH																												
50th Percentile: 31 MPH																												
85th Percentile: 38 MPH																												
95th Percentile: 41 MPH																												
10 MPH Pace Speed: 30-39 MPH																												
Number in Pace: 69																												
Percent of Vehicles > 30 MPH: 50.7%																												
Number of Vehicles > 30 MPH: 73																												
Percent of Vehicles > 30 MPH: 53.7%																												
Mean Speed(Average): 30 MPH																												
Stats																												

15th Percentile: 23 MPH
50th Percentile: 31 MPH
85th Percentile: 38 MPH
95th Percentile: 41 MPH
10 MPH Pace Speed: 30-39 MPH
Number in Pace: 69
Percent in Pace: 50.7%
Number of Vehicles > 30 MPH: 73
Percent of Vehicles > 30 MPH: 53.7%
Mean Speed(Average): 30 MPH

Site Code: 657
02797ASPEED

TDC
Transportation Data Corporation
P.O. Box 734 Natick, MA 01760
Office: 508-651-1610 Fax: 508-651-1229

Spectacle Hill Road
north of Site Driveway
City, State: Bolton, MA
Client: DJK/D. Kelly
SB

Start Time	1	15	20	25	30	35	40	45	50	55	60	65	70	999	Total	Ave. Speed
01/08/04	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
01:00	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	30
02:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
03:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
04:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
05:00	0	0	0	1	1	3	0	0	0	0	0	0	0	0	5	37
06:00	0	0	0	1	2	1	0	0	0	0	0	0	0	0	4	31
07:00	0	0	0	1	2	2	1	0	1	0	0	0	0	0	11	34
08:00	0	0	0	0	0	3	3	0	0	0	0	0	0	0	9	35
09:00	0	0	0	0	2	1	1	0	0	0	0	0	0	0	4	35
10:00	0	0	0	1	3	2	0	0	0	0	0	0	0	0	6	32
11:00	0	1	1	1	2	1	0	0	0	0	0	0	0	0	6	29
12 PM	1	0	2	3	3	1	0	0	0	0	0	0	0	0	10	26
13:00	0	0	0	3	1	4	0	0	0	0	0	0	0	0	8	32
14:00	0	0	0	2	3	2	1	0	0	0	0	0	0	0	9	32
15:00	0	1	2	2	4	0	1	0	0	0	0	0	0	0	11	31
16:00	0	0	1	4	5	2	0	0	0	0	0	0	0	0	16	33
17:00	0	0	1	5	3	2	0	0	0	0	0	0	0	0	12	32
18:00	0	0	1	2	1	0	0	0	0	0	0	0	0	0	4	26
19:00	0	0	0	3	0	0	0	0	0	0	0	0	0	0	6	30
20:00	0	0	0	0	2	0	0	0	0	0	0	0	0	0	4	26
21:00	1	0	1	0	0	0	0	0	0	0	0	0	0	0	2	10
22:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
23:00	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Total	2	3	14	29	34	30	14	1	1	0	0	0	0	0	128	*
Percent	1.6%	2.3%	10.9%	22.7%	26.6%	23.4%	10.9%	0.8%	0.8%	0.0%	0.0%	0.0%	0.0%	0.0%		

15th Percentile : 24 MPH
 50th Percentile : 32 MPH
 85th Percentile : 39 MPH
 95th Percentile : 43 MPH
 10 MPH Pace Speed : 26-35 MPH
 Number in Pace : 64
 Percent in Pace : 50.0%
 Number of Vehicles > 30 MPH : 73
 Percent of Vehicles > 30 MPH : 57.0%
 Mean Speed(Average) : 32 MPH

TDC
 Transportation Data Corporation
 P.O. Box 734 Natick, MA 01760
 Office: 508-651-1610 Fax: 508-651-1229

Site Code: 657
 02797ASPEED

Spectacle Hill Road
 north of Site Driveway
 City, State: Bolton, MA
 Client: DJK/D. Kelly

20/

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	Dermot Kelly, PE, PTOE	Intersection	Century Mill at Hudson Road
Agency/Co.	DJK Associates, Inc.	Jurisdiction	Bolton, MA
Analysis Time Period	Morning Peak Hour	Analysis Year	2005 Existing
Project Description 657 GCG, Bolton, MA			
West Street: Century Mill Road			
North/South Street: Hudson Road			
Intersection Orientation: North-South			
Study Period (hrs): 0.25			

Vehicle Volumes and Adjustments									
Major Street					Minor Street				
Northbound					Westbound				
Southbound					Eastbound				
Volume	8	154	0	0	0	26	0	0	71
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	8	167	0	0	0	28	0	0	77
Percent Heavy Vehicles	0	--	--	0	0	0	0	0	0
Median Type	Undivided								
RT Channelized	0	0	0	0	0	0	0	0	0
lanes	0	1	0	0	0	1	0	0	0
Configuration	LT								TR
Upstream Signal	0	0	0	0	0	0	0	0	0
Major Street					Minor Street				
Northbound					Westbound				
Southbound					Eastbound				
Volume	1	2	3	4	5	T	R	6	
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	8	167	0	0	0	205	8		
Percent Heavy Vehicles	0	--	--	0	--	--	--	--	--
Median Type	Undivided								
RT Channelized	0	0	0	0	0	0	0	0	0
lanes	0	1	0	0	0	1	0	0	0
Configuration	LT								TR
Upstream Signal	0	0	0	0	0	0	0	0	0

Delay, Queue Length, and Level of Service									
Approach					Approach				
NB					SB				
Westbound					Eastbound				
Movement	1	4	7	8	9	10	11	12	
Lane Configuration	LT						LR		
Flow (vph)	8						105		
Flow (m) (vph)	1369						762		
Delay	0.01						0.14		
5% queue length	0.02						0.48		
Control Delay	7.6						10.5		
LOS	A						B		
Approach Delay	--	--					10.5		
Approach LOS	--	--					B		

TWO-WAY STOP CONTROL SUMMARY

General Information		Site Information	
Analyst	Dermot Kelly, PE, PTOE	Intersection	Century Mill at Hudson Road
Agency/Co.	DJK Associates, Inc.	Jurisdiction	Bolton, MA
Date Performed	8/25/05	Analysis Year	2005 Existing
Analysis Time Period	Evening Peak Hour		
Project Description	657 GCG, Bolton, MA		
West Street: Century Mill Road	North/South Street: Hudson Road		
Intersection Orientation: North-South	Study Period (hrs): 0.25		

Vehicle Volumes and Adjustments			
Major Street	Northbound	Southbound	
Volume	54	208	0
Peak-Hour Factor, PHF	0.92	0.92	0.92
Hourly Flow Rate, HFR	58	226	0
Percent Heavy Vehicles	0	--	0
Median Type	Undivided		
Channelized			
Notes	0	0	0
Configuration	LT		
Upstream Signal	0		

Minor Street		Westbound		Eastbound	
Volume	7	8	9	10	11
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	0	0	0	0	0
Percent Heavy Vehicles	0	0	0	0	0
Percent Grade (%)	0				
Shared Approach	N			N	
Storage		0		0	
Channelized			0		0
Notes		0	0	0	0
Configuration					LR

Delay, Queue Length, and Level of Service		SB		Westbound		Eastbound	
Approach	NB	1	4	7	8	9	10
Movement	LT						LR
Volume (vph)	58						31
Queue Length (m) (vph)	1416						663
Control Delay	0.04						0.05
5% queue length	0.13						0.15
Control Delay	7.7						10.7
OS	A						B
Approach Delay	--						10.7
Approach LOS	--						B

TWO-WAY STOP CONTROL SUMMARY

3/10

General Information

Analyst: Dermot Kelly, PE, PTOE
 Agency/Co.: DJK Associates, Inc.
 Date Performed: 8/26/05
 Analysis Time Period: Morning Peak Hour

Site Information

Intersection: Century Mill at Hudson Road
 Jurisdiction: Bolton, MA
 Analysis Year: 2010 No Build

Vehicle Volumes and Adjustments

Project Description: 657 GCG, Bolton, MA	East/West Street: Century Mill Road	North/South Street: Hudson Road	Intersection Orientation: North-South	Study Period (hrs): 0.25
--	-------------------------------------	---------------------------------	---------------------------------------	--------------------------

Major Street					Minor Street				
Volume	L	T	R	12	Volume	L	T	R	12
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	0	0	0	0	Hourly Flow Rate, HFR	0	0	0	84
Percent Heavy Vehicles	0	0	0	0	Percent Heavy Vehicles	0	0	0	0
Percent Grade (%)	0	0	0	0	Percent Grade (%)	0	0	0	0
Shared Approach	N				Shared Approach	N			
Storage	0				Storage	0			
T Channelized					T Channelized				0
Lanes	0				Lanes	0			0
Configuration	LT				Configuration				LR
Upstream Signal					Upstream Signal				0
Northbound					Westbound				
Volume	1	2	3	4	Volume	0			0
L	T	R	L	T	L				0
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	9	187	0	0	Hourly Flow Rate, HFR	9	203	0	0
Percent Heavy Vehicles	0	--	--	0	Percent Heavy Vehicles	0	--	--	0
Median Type	Undivided				Median Type	Undivided			
T Channelized				0	T Channelized				0
Lanes	0	1	0	0	Lanes	0	1	0	0
Configuration				LT	Configuration				TR
Upstream Signal				0	Upstream Signal				0
Northbound					Eastbound				
Volume	7	8	9	10	Volume	0			11
L	T	R	L	T	L				T
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	0	0	0	0	Hourly Flow Rate, HFR	0	0	0	78
Percent Heavy Vehicles	0	0	0	0	Percent Heavy Vehicles	0	0	0	0
Percent Grade (%)	0	0	0	0	Percent Grade (%)	0	0	0	0
Shared Approach	N				Shared Approach	N			
Storage	0				Storage	0			
T Channelized				0	T Channelized				0
Lanes	0			0	Lanes	0			0
Configuration				LR	Configuration				LR

Delay, Queue Length, and Level of Service

Approach					Approach				
SB	WB	7	8	9	Eastbound	WB	7	8	9
Movement	1	4			11	12			
Lane Configuration	LT				LR				
v (vph)	9				115				
(m) (vph)	1337				722				
tc	0.01				0.16				
95% queue length	0.02				0.56				
Control Delay	7.7				10.9				
LOS	A				B				
Approach Delay	--				10.9				
Approach LOS	--				B				

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TWO-WAY STOP CONTROL SUMMARY

4/10

General Information

Analyst	Dermot Kelly, PE, PTOE	Intersection	Century Mill at Hudson Road
Agency/Co.	DJK Associates, Inc.	Jurisdiction	Bolton, MA
Date Performed	8/26/05	Analysis Year	2010 No Build
Analysis Time Period	Evening Peak Hour		

Project Description 657 GCG, Bolton, MA

West Street: Century Mill Road

North/South Street: Hudson Road

Intersection Orientation: North-South

Study Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street	Northbound	Southbound
--------------	------------	------------

Movement	1	2	3	4	5	6
----------	---	---	---	---	---	---

Volume	60	240	0	0	177	18
--------	----	-----	---	---	-----	----

Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
-----------------------	------	------	------	------	------	------

Hourly Flow Rate, HFR	65	260	0	0	192	19
-----------------------	----	-----	---	---	-----	----

Percent Heavy Vehicles	0	--	--	0	--	--
------------------------	---	----	----	---	----	----

Median Type	Undivided					
-------------	-----------	--	--	--	--	--

Left Channelized	0	0	0	0	0	0
------------------	---	---	---	---	---	---

Lanes	0	1	0	0	1	0
-------	---	---	---	---	---	---

Configuration	LT					TR
---------------	----	--	--	--	--	----

Upstream Signal		0			0	
-----------------	--	---	--	--	---	--

Minor Street	Westbound	Eastbound
--------------	-----------	-----------

Movement	7	8	9	10	11	12
----------	---	---	---	----	----	----

L	T	R	L	T	R	
---	---	---	---	---	---	--

Volume	0	0	0	0	0	18
--------	---	---	---	---	---	----

Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
-----------------------	------	------	------	------	------	------

Hourly Flow Rate, HFR	0	0	0	15	0	19
-----------------------	---	---	---	----	---	----

Percent Heavy Vehicles	0	0	0	0	0	0
------------------------	---	---	---	---	---	---

Percent Grade (%)	0				0	
-------------------	---	--	--	--	---	--

Shared Approach		N			N	
-----------------	--	---	--	--	---	--

Storage		0			0	
---------	--	---	--	--	---	--

Left Channelized			0			0
------------------	--	--	---	--	--	---

Lanes	0	0	0	0	0	0
-------	---	---	---	---	---	---

Configuration					LR	
---------------	--	--	--	--	----	--

Delay, Queue Length, and Level of Service

Approach	NB	SB	Westbound			Eastbound
----------	----	----	-----------	--	--	-----------

Movement	1	4	7	8	9	10
----------	---	---	---	---	---	----

Lane Configuration	LT					LR
--------------------	----	--	--	--	--	----

v (vph)	65					34
---------	----	--	--	--	--	----

(m) (vph)	1372					609
-----------	------	--	--	--	--	-----

vc	0.05					0.06
----	------	--	--	--	--	------

95% queue length	0.15					0.18
------------------	------	--	--	--	--	------

Control Delay	7.8					11.3
---------------	-----	--	--	--	--	------

LOS	A					B
-----	---	--	--	--	--	---

Approach Delay	--	--				11.3
----------------	----	----	--	--	--	------

Approach LOS	--	--				B
--------------	----	----	--	--	--	---

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TWO-WAY STOP CONTROL SUMMARY

5/10

General Information

Analyst: Dermot Kelly, PE, PTOE
 Agency/Co.: DJK Associates, Inc.
 Date Performed: 9/6/05
 Analysis Time Period: Morning Peak Hour

Site Information

Intersection: Century Mill at Hudson Road
 Jurisdiction: Bolton, MA
 Analysis Year: 2010 Build

Vehicle Volumes and Adjustments

Project Description: 657 GCG, Bolton, MA	West Street: Century Mill Road	North/South Street: Hudson Road	Intersection Orientation: North-South
			Study Period (hrs): 0.25

Major Street				Minor Street			
Northbound		Southbound		Westbound		Eastbound	
1	2	3	4	5	6	7	8
L	T	R	L	T	R	L	T
15	187	0	0	36	0	0	97
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	16	203	0	0	39	0	105
Percent Heavy Vehicles	0	--	--	0	0	0	0
Median Type				Undivided			
RT Channelized		0		0		0	
Lanes		0		0		1	
Configuration		LT				TR	
Upstream Signal		0				0	

Major Street				Minor Street			
Northbound		Southbound		Westbound		Eastbound	
1	2	3	4	5	6	7	8
L	T	R	L	T	R	L	T
0	0	0	0	36	0	0	97
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	0	0	0	0	39	0	105
Percent Heavy Vehicles	0	0	0	0	0	0	0
Percent Grade (%)				0			
Shared Approach		N				N	
Storage		0				0	
RT Channelized		0		0		0	
Lanes		0		0		0	
Configuration						LR	

Delay, Queue Length, and Level of Service

Approach				Westbound				Eastbound			
NB		SB		9		10		11		12	
Lane Configuration		LT						LR			
/ (vph)		16						144			
(m) (vph)		1333						714			
Average		0.01						0.20			
95% queue length		0.04						0.75			
Control Delay		7.7						11.3			
LOS		A						B			
Approach Delay		--						11.3			
Approach LOS		--						B			

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TWO-WAY STOP CONTROL SUMMARY

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General Information		Site Information	
Analyst	Dermot Kelly, PE, PTOE	Intersection	Century Mill at Hudson Road
Agency/Co.	DJK Associates, Inc.	Jurisdiction	Bolton, MA
Date Performed	9/6/05	Analysis Year	2010 Build
Analysis Time Period	Evening Peak Hour		
Project Description 657 GCG, Bolton, MA			
East/West Street: Century Mill Road		North/South Street: Hudson Road	
Intersection Orientation: North-South		Study Period (hrs): 0.25	

Vehicle Volumes and Adjustments											
Major Street						Minor Street					
Northbound			Southbound			Westbound			Eastbound		
1	2	3	4	5	6	7	8	9	10	11	12
L	T	R	L	T	R	L	T	R	L	T	R
Volume	82	240	0	0	26	Volume	0	0	0	0	31
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	89	260	0	0	28	Hourly Flow Rate, HFR	0	0	0	0	33
Percent Heavy Vehicles	0	--	--	0	--	Percent Heavy Vehicles	0	0	0	0	0
Median Type						Undivided					
RT Channelized						RT Channelized					
0						0					
Lanes						Lanes					
0						0					
Configuration						Configuration					
LT						TR					
Upstream Signal						0					

Delay, Queue Length, and Level of Service											
NB			SB			Westbound			Eastbound		
1	4	7	1	4	7	8	9	10	11	12	
LT	LT		LT	LT				LR	LR		
89	1361		89	1361				53	603		
(m) (vph)	(m) (vph)		(m) (vph)	(m) (vph)				53	603		
	0.07			0.07				0.09	0.09		
5% queue length	0.21			0.21				0.29	0.29		
Control Delay	7.8			7.8				11.5	11.5		
OS	A			A				B	B		
Approach Delay	--			--				11.5	11.5		
Approach LOS	--			--				B	B		

TWO-WAY STOP CONTROL SUMMARY

General Information

Site Information

Analyst	Dermot Kelly, PE, PTOE	Intersection	Century Mill at North Site Dr.
Agency/Co.	DJK Associates, Inc.	Jurisdiction	Bolton, MA
Date Performed	9/6/05	Analysis Year	2010 Build
Analysis Time Period	Morning Peak Hour		
Project Description 657 GCG, Bolton, MA			
West Street: Century Mill Road			
North/South Street: North Site Drive			
Intersection Orientation: East-West			
Study Period (hrs): 0.25			

Vehicle Volumes and Adjustments

Major Street				Eastbound				Westbound			
Movement	1	2	3	4	5	6		Movement	1	2	3
Volume (veh/h)	L	T	R	L	T	R		Volume (veh/h)	L	T	R
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		Peak-hour factor, PHF	0.92	0.92	0.92
Hourly Flow Rate (veh/h)	0	184	2	9	19	0		Hourly Flow Rate (veh/h)	0	184	2
Portion of heavy vehicles, P _{HV}	0	--	--	0	--	--		Portion of heavy vehicles, P _{HV}	0	--	--
Median type	Undivided							Median type	Undivided		
T Channelized?	0	0	0	0	1	0		T Channelized?	0	0	0
Lanes	0	1	0	0	1	0		Lanes	0	1	0
Configuration			TR		LT			Configuration			LT
Stream Signal		0			0			Stream Signal		0	

Minor Street				Northbound				Southbound			
Movement	7	8	9	10	11	12		Movement	7	8	9
Volume (veh/h)	L	T	R	L	T	R		Volume (veh/h)	L	T	R
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92		Peak-hour factor, PHF	0.92	0.92	0.92
Hourly Flow Rate (veh/h)	7	0	28	0	0	0		Hourly Flow Rate (veh/h)	7	0	28
Portion of heavy vehicles, P _{HV}	0	0	0	0	0	0		Portion of heavy vehicles, P _{HV}	0	0	0
Percent grade (%)	0	0	0	0	0	0		Percent grade (%)	0	0	0
Shared approach	N			N				Shared approach	N		
Storage	0			0				Storage	0		
T Channelized?				0				T Channelized?			
Lanes	0			0				Lanes	0		
Configuration		LR						Configuration		LR	

Control Delay, Queue Length, Level of Service

Approach				WB				Northbound				Southbound			
Movement	1	4	7	8	9	10	11	12							
Lane Configuration	LT	LR													
Volume, v (vph)	9	35													
Capacity, c _m (vph)	1401	841													
v/c ratio	0.01	0.04													
Queue length (95%)	0.02	0.13													
Control Delay (s/veh)	7.6	9.5													
LOS	A	A													
Approach delay (s/veh)	--	--													
Approach LOS	--	--													

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TWO-WAY STOP CONTROL SUMMARY

General Information

Site Information

Analyst	Dermot Kelly, PE, PTOE	Intersection	Century Mill at North Site Dr.
Agency/Co.	DJK Associates, Inc.	Jurisdiction	Bolton, MA
Date Performed	9/6/05	Analysis Year	2010 Build
Analysis Time Period	Evening Peak Hour		
Project Description 657 GCG, Bolton, MA			
East/West Street: Century Mill Road			
North/South Street: North Site Drive			
Intersection Orientation: East-West			
Study Period (hrs): 0.25			

Vehicle Volumes and Adjustments

Major Street				Eastbound				Westbound			
Movement	1	2	3	4	5			6	R		
Volume (veh/h)	L	T	R	L	T			78			
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92			0.92			
Hourly Flow Rate (veh/h)	0	34	8	32	84			0			
Proportion of heavy vehicles, P _{HV}	0	--	--	0	--			--			
Median type	Undivided										
T Channelized?			0	0	0			0			
Lanes	0	1	0	0	1			0			
Configuration			TR	LT							
Stream Signal		0			0						
Minor Street				Northbound				Southbound			
Movement	7	8	9	10	11	12					
Volume (veh/h)	L	T	R	L	T	R					
Peak-hour factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92					
Hourly Flow Rate (veh/h)	5	0	18	0	0	0					
Proportion of heavy vehicles, P _{HV}	0	0	0	0	0	0					
Percent grade (%)	0										
Shared approach		N			N						
Storage		0			0				0		
T Channelized?			0			0				0	
Lanes	0	0	0	0	0	0			0	0	
Configuration		LR									

Control Delay, Queue Length, Level of Service

Approach				WB				Northbound				Southbound			
Movement	1	4	7	8	9	10	11	12							
Lane Configuration	LT	LT		LR											
Volume, v (vph)	32	32		24											
Capacity, c _m (vph)	1580	1580		976											
svc ratio	0.02	0.02		0.02											
Queue length (95%)	0.06	0.06		0.08											
Control Delay (s/veh)	7.3	7.3		8.8											
LOS	A	A		A											
Approach delay (s/veh)	--	--		8.8											
Approach LOS	--	--		A											

TWO-WAY STOP CONTROL SUMMARY

General Information

Site Information

Analyst Agency/Co. Date Performed Analysis Time Period	Dermot Kelly, PE, PTOE DJK Associates, Inc. 9/6/05 Weekday Morning Peak Hour	Intersection Jurisdiction Analysis Year	Spectacle Hill at W. Site Dr. Bolton, MA 2010 Build
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Project Description 657 GCG, Bolton, MA

West Street: West Site Drive

North/South Street: Spectacle Hill Road

Intersection Orientation: North-South

Study Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street Northbound Southbound

Volume	LT	TR	LR	LT	TR	LR
1	2	3	4	5	6	6
L	T	R	L	T	R	R
0	7	3	2	17	0	0
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	0	7	3	2	18	0
Percent Heavy Vehicles	0	--	0	--	--	--
Median Type	Undivided					
Left Channelized	0	0	0	0	0	0
Right Channelized	0	0	0	0	0	0
Left Turn Lanes	0	0	0	0	0	0
Right Turn Lanes	0	0	0	0	0	0
Stream Signal	0	0	0	0	0	0

Minor Street Westbound Eastbound

Volume	LT	TR	LR	LT	TR	LR
1	2	3	4	5	6	6
L	T	R	L	T	R	R
10	0	5	0	0	0	0
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	10	0	5	0	0	0
Percent Heavy Vehicles	0	0	0	0	0	0
Percent Grade (%)	0	0	0	0	0	0
Right Approach	N	N	N	N	N	N
Storage	0	0	0	0	0	0
Left Channelized	0	0	0	0	0	0
Right Turn Lanes	0	0	0	0	0	0
Stream Signal	0	0	0	0	0	0

Delay, Queue Length, and Level of Service

Approach NB SB Westbound Eastbound

Configuration	LT	LR	LT	LR	LT	LR
1	4	7	8	9	10	11
12	12	12	12	12	12	12
Left Turn Lane Configuration	LT	LR	LT	LR	LT	LR
15	15	15	15	15	15	15
(m) (vph)	1623	1623	1623	1623	1623	1623
Control Delay	0.00	0.00	0.00	0.00	0.00	0.00
5% queue length	0.00	0.00	0.00	0.00	0.00	0.00
Control Delay	7.2	7.2	7.2	7.2	7.2	7.2
CS	A	A	A	A	A	A
Approach Delay	--	--	--	--	--	--
Approach LOS	--	--	--	--	--	--

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TWO-WAY STOP CONTROL SUMMARY

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General Information

Site Information

Analyst Dermot Kelly, PE, PTOE	Intersection Spectacle Hill at W. Site Dr.
Agency/Co. DJK Associates, Inc.	Jurisdiction Bolton, MA
Analysis Time Period Weekday Evening Peak Hour	Analysis Year 2010 Build
Project Description 657 GCG, Bolton, MA	
West Street: West Site Drive	North/South Street: Spectacle Hill Road
Intersection Orientation: North-South	Study Period (hrs): 0.25

Vehicle Volumes and Adjustments

Major Street						Minor Street					
Northbound			Southbound			Westbound			Eastbound		
1	2	3	4	5	6	7	8	9	10	11	12
L	T	R	L	T	R	L	T	R	L	T	R
Volume	0	13	11	5	18	6	0	3	0	0	0
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	0	14	11	5	19	6	0	3	0	0	0
Percent Heavy Vehicles	0	--	--	0	--	0	--	--	--	--	--
Median Type						Undivided					
RT Channelized			0			0			0		
Lanes			1			0			1		
Configuration			TR			LT			0		
Upstream Signal			0			0			0		

12	R	0	0	0	0	0	0	0	0	0	0
Volume	6	0	3	0	0	0	0	0	0	0	0
Peak-Hour Factor, PHF	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Hourly Flow Rate, HFR	6	0	3	0	0	0	0	0	0	0	0
Percent Heavy Vehicles	0	0	0	0	0	0	0	0	0	0	0
Percent Grade (%)	0	0	0	0	0	0	0	0	0	0	0
Flared Approach	N								N		
Storage		0							0		
RT Channelized			0							0	
Lanes	0	0	0	0	0	0	0	0	0	0	0
Configuration		LR									

Delay, Queue Length, and Level of Service

Approach			SB			Westbound			Eastbound		
Movement			4	7	8	LR			11		
Lane Configuration			LT			9			10		
V (vph)			5			9					
(m) (vph)			1603			994					
Vc			0.00			0.01					
5% queue length			0.01			0.03					
Control Delay			7.3			8.7					
LOS			A			A					
Approach Delay			--			8.7					
Approach LOS			--			A					

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